ABBREVIATIONS				
E)	EXISTING	lb(s)		
0	PER AT	LLH	LONG LEG HORIZONTAL LONG LEG VERTICAL	
AB	ANCHOR BOLT	LONG	LONGITUDINAL	
ABV ADDL	ABOVE ADDITIONAL	LRFD LT	LOAD RESISTANCE FACTOR DESIGN	
٩DJ	ADJUSTABLE	LT WT	LIGHT WEIGHT	
AESS AFF	ARCH. EXPOSED STRUCT. STEEL ABOVE FINISHED FLOOR	LTL LVL	LINTEL LAMINATED VENEER LUMBER	
AHJ	AUTHORITY HAVING JURISDICTION	MAS	MASONRY	
AHU	AIR HANDLING UNIT	MAT	MATERIAL(S)	
ALT ALUM	ALTERNATE ALUMINUM	MAX MECH	MAXIMUM MECHANICAL	
APPROX		MED	MEDIUM	
ARCH		MEP MEZZ	MECHANICAL/ELECTRICAL/PLUMBING MEZZANINE	
ASD BD	ALLOWABLE STRESS DESIGN BOARD	MEZZ	MANUFACTURE / MANUFACTURER	
3L	BRICK LEDGE	MIN	MINIMUM	
BLDG BLKG	BUILDING BLOCKING	MISC ML	MISCELLANEOUS MICROLLAM	
BM	BEAM	MTL	METAL	
30	BOTTOM OF	N	NORTH	
BOD BOTT	BASIS OF DESIGN BOTTOM	N/S NO	NEAR SIDE NUMBER	
BRG	BEARING	NOM	NOMINAL	
SMT	BASEMENT	NTS	NOT TO SCALE	
TWN ANT	BETWEEN CANTILEVER	oc OD	ON CENTER OUTSIDE DIAMETER	
;F	COLD-FORMED	OF	OUTSIDE FACE	
SIP	CAST-IN-PLACE	OH	OVERHEAD / OPPOSITE HAND	
CJP	CONTROL JOINT COMPLETE JOINT PENETRATION	OPNG OPP	OPENING OPPOSITE	
Ľ	CENTER LINE	OSB	ORIENTED STRAND BOARD	
	CEILING	PAF	POWDER ACTUATED FASTENER	
CLR MU	CLEAR CONCRETE MASONRY UNIT	PC PEN	PRECAST PENETRATION	
NTRD	CENTERED	PERIM	PERIMETER	
	COLUMN	PERP PJP	PERPENDICULAR PARTIAL JOINT PENETRATION	
ONC ONN	CONCRETE CONNECTION	PJP	PARTIAL JOINT PENETRATION PLATE	
ONST	CONSTRUCTION	PLBG	PLUMBING	
ONT	CONTINUOUS COORDINATE	plf PLWD	POUNDS PER LINEAL FOOT PLYWOOD	
/ DL	DEAD LOAD	psf	POUNDS PER SQUARE FOOT	
BL	DOUBLE	psi	POUNDS PER SQUARE INCH	
IA IAG	DIAMETER DIAGONAL	PT RAD	POST-TENSIONED/PRESSURE TREATED RADIUS	
IM	DIMENSION	RE	REFERENCE / REFER TO	
IV	DIVISION	REINF	REINFORCED(ING)	
N TL	DOWN DETAIL	REQD REV	REQUIRED REVISION	
WG	DRAWING	RO	ROUGH OPENING	
WL	DOWEL	RTU	ROOF TOP UNIT	
/F	EARTHQUAKE/SEISMIC LOAD / EAST EACH FACE	S S/SL	SOUTH SNOW LOAD	
/S	EACH SIDE	SA	SUPPLY AIR	
A IFS		SC SCHED	SLIP-CRITICAL	
J	EXTERIOR INSUL. FINISH SYSTEM EXPANSION JOINT	SECT	SCHEDULE SECTION	
ELEC	ELECTRIC(AL)	SF	SQUARE FEET	
LEV MBED	ELEVATION EMBEDMENT	SHT SIM	SHEET SIMILAR	
NCL	ENCLOSURE	SOG	SLAB-ON-GRADE	
NGR	ENGINEER	SPEC	SPECIFICATION(S)	
O OR	EDGE OF ENGINEER OF RECORD	SQ SS	SQUARE STAINLESS STEEL	
Q	EQUAL	STD	STANDARD	
QUIP	EQUIPMENT	STIFF	STIFFENER	
ST XP	ESTIMATE EXPANSION	STL STOR	STEEL STORAGE	
XT	EXTERIOR	STRUCT	STRUCTURAL	
/S F	FAR SIDE	SYM T&B	SYMMETRICAL TOP AND BOTTOM	
FE	FINISH FLOOR FINISHED FLOOR ELEVATION	T&B T&G	TOP AND BOTTOM TONGUE AND GROOVE	
IN	FINISH(ED)	T/C	TENSION-CONTROL	
NDN O	FOUNDATION FACE OF	TBD THK	TO BE DETERMINED THICK(NESS)	
T	FACE OF FEET/FOOT	THR	THROUGH	
TG	FOOTING	TL	TOTAL LOAD	
V A	FIELD VERIFY GAUGE	TO TOBL	TOP OF TOP OF BRICK LEDGE	
ALV	GAUGE	TOC	TOP OF CONCRETE	
C	GENERAL CONTRACTOR	TOF		
YP YP BD	GYPSUM GYPSUM BOARD	TOM TOP	TOP OF MASONRY TOP OF PILASTER / PIER	
AS	HEADED ANCHOR STUD	TOS	TOP OF STEEL / SLAB	
ORZ		TOW		
<u>SS</u> T	HOLLOW STRUCTURAL SECTION HEIGHT	TRANSV TS	TRANSVERSE TUBE STEEL	
VAC	HEATING/VENTILATING/AIR CONDITIONING	TYP	TYPICAL	
BC	INTERNATIONAL BUILDING CODE	ULT		
)	INSIDE DIAMETER INSIDE FACE	UNO VERT	UNLESS NOTED OTHERWISE VERTICAL	
1	INCHES / INCH	VIF	VERIFY IN FIELD	
		W	WIDE/WIDTH WIND LOAD	
ISUL	INSULATION / INSULATING INTERIOR	W/	WIND LOAD WITH	
NT	JOINT	W/C	WATER/CEMENT	
ST	JOIST KIPS	W/O WD	WITHOUT WOOD	
f	KIPS KIPS PER LINEAL FOOT	WD	WIDE-FLANGE	
sf si	KIPS PER SQUARE FOOT KIPS PER SQUARE INCH	WP WT	WORK POINT WEIGHT	

SYMBOLS	DESIGN CRITERIA	MAT
	1. CODES & STANDARDS: A. INTERNATIONAL BUILDING CODE 2021 EDITION	1. CONCRETE: A. CONCRETE SHALL BE MADE WIT
NORTH ARROW	<ul> <li>B. GREELEY, COLORADO CODE AMENDMENTS</li> <li>C. ASCE/SEI 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES</li> <li>D. ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE</li> </ul>	28 DAY COMPRESSIVE STRENG 1. FOOTINGS 2. FOUNDATION WALLS & PILA
SECTION DETAIL	<ul> <li>E. ANSI/AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS</li> <li>F. ANSI/AWC NDS-2018 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION</li> </ul>	<ol> <li>INTERIOR SLABS ON GRADE</li> <li>SITE TRASH ENCLOSURE S</li> </ol>
SECTION DETAIL	<ul> <li>G. TMS 402/602-2016 BUILDING CODE AND SPECIFICATIONS FOR MASONRY STRUCTURES</li> <li>2. GRAVITY LOADS USED IN DESIGN:</li> </ul>	<ul> <li>B. RE: CIVIL FOR CONCRETE MIXTURE</li> <li>C. PROPORTIONS OF MATERIALS I CONSISTENCY TO PERMIT CONTINUES</li> </ul>
PLAN / GENERAL	A.ROOF SNOW LOAD30 psf + DRIFTING REQUIREMENTSB.SUPERIMPOSED ROOF DEAD LOAD15 psf	CONDITIONS OF PLACEMENT TO SELECT AN APPROPRIATE SLUM
DETAIL SI SHEET REFERENCE	C. FLOOR LIVE LOAD 50 psf D. OFFICE PARTITION LIVE LOAD 15 psf E. LOBBY, 1 <sup>ST</sup> FLOOR CORRIDOR LIVE LOAD 100 psf	D. TYPE I/II PORTLAND CEMENT SH E. FOR CONCRETE MIXTURES USE YARD. WHEN FLY ASH IS USED
	F.MECHANICAL EQUIPMENT LOADSRE: MECHANICAL DRAWINGSG.SNOW LOAD DESIGN CRITERIA	15% NOR MORE THAN 25% BY W F. FOR CONCRETE PLACED BY PU
ELEVATION DETAIL	<ol> <li>GROUND SNOW LOAD, Pg = 30 psf</li> <li>FLAT ROOF SNOW LOAD, Pf = 21 psf</li> <li>SNOW EXPOSURE FACTOR, Ce = 1.0</li> </ol>	FACILITATE PUMPING. ENTRAIN INCLUDED HEREIN. G. WHERE AIR CONTENT IS INDICA
	<ol> <li>SNOW LOAD IMPORTANCE FACTOR, Is = 1.0</li> <li>THERMAL FACTOR, Ct = 1.0</li> </ol>	INCLUDES BOTH ENTRAINED AN INDICATED IS BASED ON A NOM
	3. WIND LOAD DESIGN CRITERIA: A. BUILDING RISK CATEGORY II	REQUIRED AIR CONTENT PER A H. THE ADDITION OF ENTRAINED A CAN DEMONSTRATE TO THE AR
GRIDLINE - NEW	<ul> <li>B. BASIC DESIGN WIND SPEED = 115 mph</li> <li>C. EXPOSURE CATEGORY C</li> </ul>	FINISH WITHOUT BLISTERS. I. VERIFY ALKALINITY OF CONCRE
GRIDLINE - EXISTING	4. SEISMIC LOAD DESIGN CRIERIA: A. BUILDING RISK CATEGORY II	COMPATIBLE WITH FLOOR SYST J. NO CHLORIDE ADMIXTURES SHA K. ALL REINFORCING BARS SHALL
	<ul> <li>B. SEISMIC IMPORTANCE FACTOR, le = 1.0</li> <li>C. SITE CLASS D</li> </ul>	SHALL CONFORM TO ASTM A185 L. CONCRETE PROTECTION FOR F
	<ul> <li>D. SITE SPECTRAL RESPONSE ACCELERATIONS:</li> <li>1. Ss = 0.153</li> <li>2. S1 = 0.051</li> </ul>	<ol> <li>CONCRETE POURED AGAINS</li> <li>CONCRETE POURED IN FORM #5 BARS OR SMALLER</li> </ol>
INDICATES XXXXXXX TYPICAL NOTE	3. SDs = 0.163 4. SD1 = 0.082	BARS LARGER THAN #5 3. SLABS AND WALLS NOT EXPO
	<ul> <li>E. SEISMIC DESIGN CATEGORY B</li> <li>F. BASIC SEISMIC FORCE RESISTING SYSTEM: WOOD-FRAMED SHEARWALLS WITH WOOD PANELS</li> <li>G. RESPONSE MODIFICATION FACTOR, R = 6.5</li> </ul>	M. NO SPLICES OF REINFORCEMEN DETAILED OR AUTHORIZED BY 1 48 BAR DIAMETERS, UNLESS NO
SPAN DIRECTION OF DECK/SHEATHING	<ul> <li>H. SEISMIC RESPONSE COEFFICIENT, Cs = 0.0251</li> <li>I. ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE</li> </ul>	SIDE AND END LAPS, BUT NOT L OR PROVIDE CORNER BARS OF
	<ul> <li>5. LATERAL FORCE RESISTING SYSTEM DESCRIPTION:</li> <li>A. LATERAL FORCE RESISTANCE AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED</li> </ul>	<ul> <li>N. DETAIL BARS IN ACCORDANCE</li> <li>REINFORCED CONCRETE.</li> <li>O. PROVIDE ALL ACCESSORIES NE</li> </ul>
	BY VERTICAL WOOD-FRAMED SHEARWALLS. B. THE WOOD ROOF DECKS SERVE AS HORIZONTAL DIAPHRAGMS THAT DISTRIBUTE LATERAL WIND AND SEISMIC	P. PLACE 2-#5 BARS (1 EACH FACE CONCRETE SLABS AND WALLS,
	FORCES TO THE VERTICAL LATERAL ELEMENTS. THE VERTICAL LATERAL ELEMENTS CARRY APPLIED LATERAL LOADS TO THE BUILDING FOUNDATIONS.	Q. CONCRETE WORK SHALL NOT H OR AS APPROVED BY THE STRU R. ALLOW FOR ADDITIONAL CONC
	<ul> <li>FOUNDATIONS:</li> <li>A. SOIL DATA WAS TAKEN FROM RECOMMENDATIONS SET FORTH IN PROJECT GEOTECHNICAL REPORT BY NINYO</li> <li>MOODE DATED OCTOBED 32, 2024 (PROJECT NO. 802044004)</li> </ul>	S. SLAB-ON-GRADE CONTROL JOIN COLUMN CENTERLINES AND RE
CLOUD REVISION	& MOORE DATED OCTOBER 23, 2024 (PROJECT NO. 803044001). REFER TO GEOTECHNICAL REPORT FOR COMPLETE SOILS INFORMATION. B. MAXIMUM TOTAL LOAD SOIL BEARING PRESSURE CAPACITY USED IN DESIGN IS 3,000 psf.	2. STRUCTURAL STEEL: A. ALL STRUCTURAL STEEL SHALL ASTM A992 (GRADE 50), PIPE SE
FOOTING/DRILLED PIER $\langle \# \rangle$		CONFORM TO ASTM A500 (GRAD B. STRUCTURAL STEEL SHALL BE
Түре	GENERAL NOTES	CONSTRUCTION MANUAL, INCLU C. ALL WELDERS SHALL HAVE EVII CURRENT CERTIFICATION.
COLUMN TYPE	<ol> <li>CONTRACTOR RESPONSIBILITY DURING CONSTRUCTION/ERECTION:</li> <li>A. THE STRUCTURE IS DESIGNED TO FUNCTION AS A COMPLETED UNIT, WITH ALL SPECIFIED ELEMENTS AND</li> </ol>	D. MINIMUM WELD SIZE SHALL NO E. CONNECTIONS MADE WITH HIG STRUCTURAL JOINTS USING A3
STEP	CONNECTIONS IN PLACE AND FULLY INSTALLED. B. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, SPECIFICATION, AND IMPLEMENTATION OF ANY AND ALL TEMPORARY BRACING OR SHORING REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND	F. ANCHOR RODS SHALL CONFOR G. FABRICATE BEAMS SUCH THAT
SLOPE 7/////	METHODS OF CONSTRUCTION AND SEQUENCES OF ERECTION. SUCH BRACING OR SHORING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY AND UNTIL ALL STRUCTURAL FRAMING AND DIAPHRAGMS	<ul> <li>H. NON-SHRINK GROUT SHALL COI 6,000 psi AT 28 DAYS.</li> <li>I. ALL STEEL OR STEEL CONNECT</li> </ul>
·//	ARE IN PLACE WITH CONNECTIONS COMPLETED. C. THE COMPLETED STRUCTURE HAS BEEN DESIGNED ONLY FOR APPLICABLE CODE-PRESCRIBED LOADS ANTICIPATED DURING ITS SERVICE LIFE AS INDICATED HEREIN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR	AN ASPHALTIC MASTIC. 3. WOOD - GENERAL:
GRAPHIC SCALE BAR	ANY ADDITIONAL ENGINEERING AND OTHER MEASURES REQUIRED TO ACHIEVE THE CONTRACTOR'S MEANS, METHODS, AND SEQUENCES OF CONSTRUCTION AND TO WITHSTAND ALL TEMPORARY CONSTRUCTION LOADS	<ul> <li>A. ALL SAWN DIMENSION LUMBER GRADE OR BETTER.</li> <li>B. TIMBER CONNECTORS SPECIFIE</li> </ul>
0' 2' 4' 8'	TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING BUT NOT LIMITED TO ERECTION LOADING AND STOCKPILING OF MATERIALS AND EQUIPMENT. 2. ALL DIMENSIONS ON STRUCTURAL DRAWINGS SHALL BE CHECKED AGAINST ARCHITECTURAL.	MANUFACTURERS MAY BE USE TO OR GREATER THAN THE CO
MATERIALS	3. STRUCTURAL DRAWINGS ARE NOT TO BE SCALED TO DETERMINE DIMENSIONAL INFORMATION, TO VERIFY OR COORDINATE ANY INFORMATION PRESENTED OR FOR ANY OTHER PURPOSE.	C. INSTALL A CONTINUOUS ROW C D. GLUE AND NAIL TOGETHER ALL E. PROVIDE FULL-HEIGHT 2x KING
	<ol> <li>WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, AND GENERAL NOTES, THE MORE STRINGENT REQUIREMENT SHALL GOVERN.</li> <li>STRUCTURAL ENGINEER'S APPROVAL MUST BE SECURED FOR ALL SUBSTITUTIONS.</li> </ol>	F. SHEATHING FOR ROOFS AND W TO THE PROVISIONS OF APA PR
CAST-IN-PLACE CONCRETE	<ol> <li>VERIFY ALL OPENINGS THROUGH FLOOR, ROOF AND WALLS WITH MECHANICAL AND ELECTRICAL CONTRACTORS.</li> <li>STRUCTURAL SPECIAL INSPECTIONS SHALL BE PROVIDED AS SPECIFIED AND IN ACCORDANCE WITH CHAPTER 17 OF</li> </ol>	G. SHEATHING THICKNESS AND FA 1. SHEARWALLS: ALL DESIGN DRAWINGS. ALL SHEARWA
PRECAST OR EXISTING CONCRETE	<ul> <li>THE INTERNATIONAL BUILDING CODE.</li> <li>8. THE STRUCTURAL ENGINEER'S PERIODIC OBSERVATIONS OR PRESENCE ON SITE DOES NOT REPLACE OR PRECLUDE THE NEED FOR ANY INDEPENDENT THIRD PARTY STRUCTURAL SPECIAL INSPECTIONS SPECIFIED HEREIN.</li> </ul>	2. NON SHEARWALLS: TYPICA SHEATHING FASTENED WIT
STEEL	9. SIGNIFICANT PERMANENT EQUIPMENT SIZES, WEIGHTS, AND LOCATIONS INDICATED ON THE DRAWINGS ARE AS PROVIDED TO THE STRUCTURAL ENGINEER DURING DESIGN. CHANGES IN SIZES, WEIGHTS, OR LOCATIONS FROM	MEMBERS. NON SHEARWA 3. ROOF DIAPHRAGMS: ONE L PANEL EDGES AND @ 12" O
CONCRETE MASONRY	THAT INDICATED MUST BE SUBMITTED IN WRITING FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER. REQUIRED SUPPORTS OR BRACES NOT SHOWN ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE EQUIPMENT SUPPLIER.	<ul><li>H. SMOOTH COMMON NAILS SHALL</li><li>I. PLACE SHEATHING WITH 8'-0" D</li></ul>
BRICK MASONRY	10. THE FOLLOWING IS A LIST OF DELEGATED DESIGN AND/OR PERFORMANCE-SPECIFIED ELEMENTS TO BE DESIGNED BY OTHERS AND PRESENTED FOR APPROVAL AS A DEFERRED SUBMITTAL.	STAGGERED. ROOF DIAPHRAG J. FASTEN ALL WOOD MEMBERS P K. LVL BEAMS AND I-JOISTS SHALL
	A. PRE-MANUFACTURED WOOD I-JOIST AND LVL FRAMING ELEMENTS	ALL PLATES, BLOCKING, BRIDGI 4. PREFABRICATED WOOD ROOF I-JOI
	CONTRACTOR SUBMITTALS	A. SUPPLIER SHALL DESIGN AND F SUPPLIER SHALL SUBMIT SHOP PROFESSIONAL ENGINEER FOR
GRAVEL FILL	1. GENERAL CONTRACTOR SHALL PROVIDE THE FOLLOWING MATERIAL SPECIFICATION AND SHOP DRAWING	PROJECT SITE. B. RE: DESIGN CRITERIA NOTES TH C. JOISTS SHALL BE DESIGNED TO
RIGID INSULATION	<ul> <li>SUBMITTALS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING</li> <li>WITH EACH APPLICABLE PORTION OF THE WORK.</li> <li>ITEMS TO BE SUBMITTED AS SPECIFIED IN THE DRAWINGS:</li> </ul>	1. MAXIMUM TOTAL LOAD DEF 2. MAXIMUM SNOW LOAD DEF
	<ul><li>A. CONCRETE DESIGN MIXTURES</li><li>B. CONCRETE STEEL REINFORCING SHOP DRAWINGS</li></ul>	<ul> <li>D. ALL JOISTS SHALL BE ERECTED TEMPORARY BRACING.</li> <li>E. ALL REQUIRED BRIDGING AND E</li> </ul>
O CONTRACTOR	C. STRUCTURAL STEEL SHOP DRAWINGS D. MASONRY MORTAR AND GROUT DESIGN MIXTURES E. MASONRY ELEMENT AND ASSEMBLY SPECIFICATIONS	5. MASONRY: A. CONCRETE MASONRY BLOCK U
ORDINATE WITH THE STRUCTURAL ENGINEER TO RESOLVE CONFLICTS PRIOR TO COMMENCING WITH CONSTRUCTION	<ol> <li>DELEGATED DESIGN AND/OR PERFORMANCE-SPECIFIED ELEMENTS TO BE DESIGNED AND PROVIDED BY THE CONTRACTOR:</li> </ol>	<ul> <li>B. MORTAR FOR STRUCTURAL WA</li> <li>C. GROUT FOR MASONRY SHALL C STRENGTH OF 2,000 psi.</li> </ul>
	[SUBMITTALS ARE TO INCLUDE SUPPORTING CALCULATIONS STAMPED AND SIGNED BY A QUALIFIED STRUCTURAL ENGINEER LICENSED IN THE STATE OF COLORADO. DRAWINGS AND CALCULATIONS ARE TO INCLUDE THE INDICATED ELEMENTS AND THEIR ATTACHMENTS TO THE MAIN BUILDING STRUCTURE.]	D. ALL MASONRY ASSEMBLIES SHA E. STANDARD HORIZONTAL STEEL
ATE THE STRUCTURAL ENGINEER AS TO THE STATUS AND IALL COORDINATE WITH THE STRUCTURAL ENGINEER TO BSERVE COMPLETED AND ONGOING PORTION OF THE	<ul> <li>A. PRE-MANUFACTURED WOOD I-JOIST AND LVL FRAMING ELEMENTS</li> <li>4. GENERAL CONTRACTOR SHALL REVIEW EACH SUBMITTAL AND CHECK FOR COORDINATION WITH OTHER WORK AND</li> </ul>	WALLS, SPACED AT 16" ON CEN F. SPECIFIED VERTICAL REINFORC USING A MAXIMUM OF 5'-0" LIFT
	FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS, AND SHALL AFFIX THEIR STAMP INDICATING APPROVAL PRIOR TO SENDING TO THE ARCHITECT AND STRUCTURAL ENGINEER. 5. SUBMITTALS THAT DO NOT BEAR THE CONTRACTOR'S APPROVAL STAMP WILL NOT BE REVIEWED AND WILL BE	PROVIDED AT THE BASE OF EAC REINFORCING MUST BE ADEQU
OF ALL REQUIRED SUBMITTALS INDICATED IN THE GENERAL FABRICATION OR CONSTRUCTION OF EACH APPLICABLE	RETURNED WITHOUT ACTION.DO NOT REPRODUCE CONTRACT DOCUMENTS, COPY STANDARD PRINTED INFORMATION, OR USE ELECTRONIC DRAWING FILES AS THE BASIS FOR SHOP DRAWINGS.	<ul><li>G. PROVIDE ONE ADDITIONAL VER</li><li>OF OPENINGS.</li><li>H. PROVIDE HORIZONTAL BOND BE</li></ul>
RAL SPECIAL INSPECTIONS AND TESTS WITH THE	6. GENERAL CONTRACTOR SHALL PROVIDE COPIES OF ALL FINAL, APPROVED, FOR CONSTRUCTION MATERIAL SPECIFICATION AND SHOP DRAWING SUBMITTALS TO THE ARCHITECT AND STRUCTURAL ENGINEER.	STRUCTURAL WALLS 6. POST-INSTALLED ANCHORS:
OWNER.		A. POST-INSTALLED ANCHORS SHA SPECIFICATIONS AND PRINTED MATERIALS.
		B. FOR EXPANSION ANCHORS INS MINIMUM COMPRESSIVE STREN

SHEET	INDEX	

SHEET #	SHEET NAME
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S2.1A	FOUNDATION/FLOOR PLAN - NORTH
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S3.1	FOUNDATION / FLOOR DETAILS
S4.1	FRAMING DETAILS

### NOTE TC

READ ALL STRUCTURAL NOTES AND COOR ANY QUESTIONS, DISCREPANCIES, OR COM OPERATIONS.

CONTRACTOR SHALL INFORM AND UPDATE SCHEDULE OF CONSTRUCTION, AND SHAL SCHEDULE PERIODIC SITE VISITS TO OBSE WORK

STRUCTURAL ENGINEER'S APPROVAL OF A NOTES SHALL BE PROCURED PRIOR TO FA PORTION OF THE WORK.

COORDINATE ALL REQUIRED STRUCTURAL INSPECTION AGENCY ENGAGED BY THE OW

> B. FOR EXPANSION ANCHORS INSTALLED INTO CONCRETE, THE CONCRETE BASE MATERIAL SHALL REACH THE REQUIRED MINIMUM COMPRESSIVE STRENGTH (F'C) SPECIFIED IN THE DRAWINGS PRIOR TO ANCHOR INSTALLATION. C. FOR ADHESIVE ANCHORS INSTALLED INTO CONCRETE, THE CONCRETE BASE MATERIAL AT THE TIME OF ANCHOR INSTALLATION SHALL HAVE A MINIMUM AGE OF 21 DAYS, A MINIMUM CONCRETE TEMPERATURE OF 50 DEGREES F, AND SHALL REACH THE REQUIRED MINIMUM COMPRESSIVE STRENGTH (F'C) SPECIFIED IN THE DRAWINGS PRIOR TO ANCHOR

INSTALLATION.

## **ATERIAL SPECIFICATIONS & NOTES**

WITH STONE AGGREGATE (NORMAL-WEIGHT CONCRETE) AND SHALL DEVELOP THE FOLLOWING NGTHS (F'C) WITH THE INDICATED DESIGN MIXTURE PROPERTIES:

3,500 psi 3.500 psi : 0.55 MAX W/C RATIO : 5% AIR ILASTERS

٩DE 3,500 psi E SLAB & FOUNDATION 4,000 psi ; 0.40 MAX W/C RATIO ; 6% AIR

XTURES TO BE USED FOR EXTERIOR SITE SLAB, PAVING, AND FLATWORK, ETC. S IN CONCRETE MIXTURES SHALL BE ESTABLISHED TO PROVIDE WORKABILITY AND ONCRETE TO BE PLACED READILY INTO FORMS AND AROUND REINFORCEMENT, UNDER

TO BE EMPLOYED, WITHOUT SEGREGATION OR EXCESSIVE BLEEDING. CONTRACTOR SHALL LUMP, AND ADMIXTURES MAY BE USED AS NEEDED TO OBTAIN ACCEPTABLE RESULTS. SHALL BE USED, UNLESS NOTED OTHERWISE.

JSED FOR FLOOR SLABS, THE MINIMUM CEMENTITIOUS CONTENT SHALL BE 540 lbs. PER CUBIC ED AS A SUPPLEMENTARY CEMENTITIOUS MATERIAL, QUANTITY SHALL NOT BE LESS THAN WEIGHT OF TOTAL CEMENTITIOUS MATERIAL.

PUMPING, THE DESIGN MIXTURE SHALL BE PROPORTIONED TO ENSURE FLOWABILITY TO AINED AIR MAY BE USED TO FACILITATE PUMPING, SUBJECT TO THE PROVISIONS AND LIMITS

ICATED ABOVE, PROVIDE AIR ENTRAINING ADMIXTURES. AIR CONTENT VALUE INDICATED AND ENTRAPPED AIR, AND MAY BE PROVIDED WITHIN A RANGE OF +/- 1.5%. AIR CONTENT OMINAL AGGREGATE SIZE OF 3/4". IF ANOTHER AGGREGATE SIZE IS TO BE USED, ADJUST R ACI 318 TABLE 19.3.3.1.

D AIR IS NOT PERMITTED IN MIXTURES TO BE USED AS FLOOR SLABS, UNLESS THE CONTRACTOR ARCHITECT THAT SUCH SLAB MIXTURES WITH ENTRAINED AIR WILL PROVIDE AN ACCEPTABLE

CRETE SLAB SURFACE, SLAB VAPOR TRANSMISSION AND SLAB FLATNESS/LEVELNESS ARE YSTEMS AND ADHESIVES PRIOR TO INSTALLING FLOORING. SHALL BE ADDED TO CONCRETE WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

ALL BE ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. WELDED WIRE REINFORCING 185 OR ASTM A497. R REINFORCEMENT (UNLESS NOTED OTHERWISE):

NST EARTH

ORMS BUT EXPOSED TO WEATHER OR EARTH 1-1/2"

XPOSED TO WEATHER OR EARTH 3/4" MENT SHALL BE MADE AND NO WELDING TO REINFORCING SHALL BE PERMITTED EXCEPT AS BY THE STRUCTURAL ENGINEER. REINFORCING LAP SPLICES ARE TO BE A MINIMUM OF NOTED OTHERWISE. WIRE FABRIC REINFORCEMENT MUST LAP ONE FULL MESH PLUS 2" AT DT LESS THAN 6", AND SHALL BE WIRED TOGETHER. MAKE ALL BARS CONTINUOUS AT CORNERS OF EQUAL SIZE AND SPACING. CE WITH THE ACI DETAILING MANUAL AND ACI BUILDING CODE REQUIREMENTS FOR

NECESSARY TO SUPPORT REINFORCING AT POSITIONS SHOWN.

CE) WITH 2'-0" PROJECTION AROUND ALL OPENINGS AND RE-ENTRANT CORNERS IN \_S, UNLESS NOTED OTHERWISE. T HAVE JOINTS IN A HORIZONTAL PLANE. ALL CONSTRUCTION JOINTS SHALL BE AS DETAILED

RUCTURAL ENGINEER. NCRETE THICKNESS TO COMPENSATE FOR STRUCTURAL MEMBER AND FORMWORK DEFLECTIONS.

OINTS ARE TO BE SPACED A MAXIMUM OF 12'-0" ON CENTER AND ARE TO COINCIDE WITH **RE-ENTRANT CORNERS.** 

ALL CONFORM TO ASTM A36, EXCEPT WIDE FLANGE SHAPES WHICH SHALL CONFORM TO SECTIONS WHICH SHALL CONFORM TO ASTM A53 (GRADE B), AND HSS SECTIONS WHICH SHALL RADE C)

3E DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC STEEL CLUDING ANSI/AISC 303 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS. EVIDENCE OF PASSING THE AWS STANDARD QUALIFICATION TESTS, AND HAVE A

NOT BE LESS THAN 3/16" CONTINUOUS FILLET WELD, UNLESS NOTED OTHERWISE. IIGH STRENGTH STEEL BOLTS SHALL CONFORM TO THE AISC SPECIFICATION FOR A325 OR A490 BOLTS. ORM TO ASTM F1554 GRADE 36 AND ARE TO BE HOOKED, UNLESS NOTED OTHERWISE.

AT ROLLING OR FABRICATION INDUCED CAMBER IS UP AFTER ERECTION. COMPLY WITH ASTM C1107 AND ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF

CTIONS PERMANENTLY EXPOSED BELOW GRADE SHALL BE COATED WITH A ZINC RICH PAINT OR

ER FOR STRUCTURAL FRAMING SHALL BE VISUALLY-GRADED, SURFACED DRY HEM FIR, NO.2

IFIED ARE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY. CONNECTORS BY OTHER SED IF THEY HAVE A CURRENT ICC-ES EVALUATION REPORT AND THEIR LOAD CAPACITY IS EQUAL CONNECTOR SPECIFIED. USE MANUFACTURER'S FURNISHED FASTENERS. V OF HORIZONTAL BLOCKING AT MID-HEIGHT OF SINGLE STORY WALLS OVER 10'-0" TALL. ALL PLIES OF BUILT-UP MEMBERS.

NG STUD AT ALL BEAM AND HEADER BEARING LOCATIONS, UNLESS NOTED OTHERWISE. ) WALLS SHALL BE APA RATED WITH EXPOSURE 1 BOND CLASSIFICATION, AND SHALL CONFORM PRP-108 OR VOLUNTARY PRODUCT STANDARD PS 1-07.

) FASTENING REQUIREMENTS SHALL BE AS FOLLOWS: GNATED SHEARWALLS SHALL HAVE THE SHEATHING THICKNESS AND NAILING SPECIFIED IN THE WALL PANEL EDGES SHALL BE SOLID BLOCKED AT INTERMEDIATE FRAMING MEMBERS. ICAL WALLS NOT DESIGNATED AS SHEARWALLS SHALL HAVE ONE LAYER OF 15/32" NITH 8D NAILS @ 6" O.C. ALONG PANEL EDGES AND @ 12" O.C. AT INTERMEDIATE FRAMING WALL PANEL EDGES NEED NOT BE BLOCKED AT INTERMEDIATE FRAMING MEMBERS. NE LAYER OF 23/32" SHEATHING FASTENED WITH 10D NAILS SPACED @ 6" O.C. ALONG

" O.C. AT INTERMEDIATE FRAMING MEMBERS.

ALL BE USED ON ALL ROOF AND WALL SHEATHING. " DIMENSION PERPENDICULAR TO SPAN OF FRAMING MEMBERS AND WITH END JOINTS AGMS SHALL NOT BE GLUED.

S PER IBC TABLE 2304.10.1, UNLESS NOTED OTHERWISE.

ALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. DGING AND OTHER RELATED ITEMS SHALL BE FURNISHED BY THE MANUFACTURER. JOISTS:

ID PROVIDE JOISTS IN ACCORDANCE WITH THE DESIGN CRITERIA OUTLINED IN THE DRAWINGS. OP DRAWINGS AND CALCULATIONS STAMPED AND SIGNED BY A COLORADO LICENSED OR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO DELIVERY TO THE

THIS SHEET AND SHEET S1.3 FOR APPLICABLE DESIGN LOADS. TO PROVIDE FOR MAXIMUM VERTICAL DEFLECTIONS AS FOLLOWS:

DEFLECTION: L/240 OF JOIST SPAN

DEFLECTION: L/360 OF JOIST SPAN ED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INCLUDING CONSIDERATIONS FOR

ID BLOCKING SHALL BE INSTALLED PRIOR TO INSTALLING SHEATHING.

K UNITS SHALL BE MEDIUM WEIGHT AND SHALL CONFORM TO ASTM C90. WALLS SHALL CONFORM TO ASTM C270.

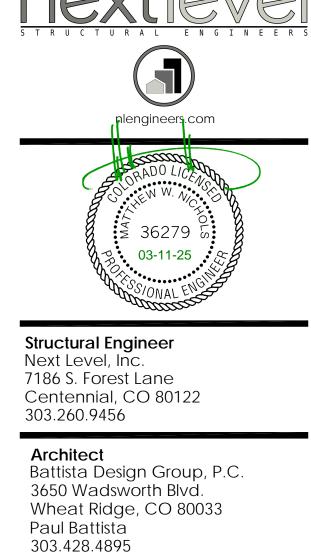
L CONFORM TO ASTM C476 AND SHALL DEVELOP A MINIMUM 28-DAY COMPRESSIVE

SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH (F'M) OF 1,500 psi IN 28 DAYS. EL LADDER MESH REINFORCING SHALL BE PROVIDED IN BED JOINTS OF STRUCTURAL MASONRY ENTER, UNLESS NOTED OTHERWISE.

DRCING SHALL EXTEND FOR THE FULL HEIGHT OF THE WALL AND SHALL BE GROUTED IN PLACE FTS. HIGH LIFT GROUTING MAY BE DONE UP TO A HEIGHT OF 15'-0" AS LONG AS CLEANOUTS ARE EACH GROUTED CELL. ALL CELLS MUST BE CLEANED PRIOR TO GROUTING, AND ALL VERTICAL QUATELY SECURED INTO POSITION. ERTICAL BAR AT THE FOLLOWING LOCATIONS: WALL CORNERS, ENDS OF WALLS, AND EACH SIDE

) BEAM REINFORCED WITH A MINIMUM OF 1-#5 CONTINUOUS BARS AT THE TOP OF ALL

SHALL BE INSTALLED IN ACCORDANCE WITH ALL MANUFACTURER'S RECOMMENDATIONS, ED INSTALLATION INSTRUCTIONS (MPII), AND SHALL BE INSTALLED ONLY INTO DRY BASE



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Principal

Matt Nichols, P.E. 303.260.9456

matt@nlengineers.com 25-001 Project Number

Description	Date
	02.11.2025

COVER SHEET, DESIGN CRITERIA & PROJECT NOTES

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a BEFCON DEVELOPMENT AND A DEVELOPMENT AND ADDRESS AND THAT A DEVELOPMENT ADDRESS ADDR	D.	,	,	4.
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B. FOR OTHER STEEL, BENTFICATION MARKINGS TO CONTORN TO ASTA STANDARDS SPECIPIED IN THE         [P]         I           2. MATERIAL VERIFICATION CONTINUES TECHNICIDATES AND CARD CARD MARKERS         [P]         []           2. MATERIAL VERIFICATION CONTINUES TECHNICIDATES AND CARD CARD MARKERS         [P]         []           3. MALEMENT CONTINUES CONTINUES AND CARD CARD MARKERS         [P]         []         []           3. MALEMENT CONTINUES CONTINUES AND CONTINUES AND CARD CARD MARKERS         [P]         []         []           4. IDENTIFICATION MARKINGS TO CONTONITO AND SATE STRUCTURE DO CONTINUES AND CARD MARKERS         [P]         []         []           6. INSTRUCTURE DO CONTINUES CONTINUES AND CONTINUES AND CARD MARKERS         []	1.			1.
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A. IDENTIFICATION MARKINGS TO COMPORT TO ANS SPECIFICATIONS IN THE CONSTRUCTION DOCUMENTS     PI     A. INSPECTION OF HIGH TISENON TOUTING:     A. NOTO FIND TOTALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUCCED.     (P)     VERPLY CONNECTED MATERIAL SHAVE BEEN DRAWN TOGETHER AND PROPERLY SNUCCED.     (P)     A. FITLY OF FILET AND GROOP WELDS     (P)     A. FITLY OF HILET AND GROOP WELDS     (P)     A. FITLY OF FILET AND GROOP WELDS     (P)     (P)     A. FITLY OF FILET AND GROOP WELDS     (P)     (P)     (P)     (P)     (C)     (C) ONTO THE FILET AND GROOP WELDS     (P)     (				
A. SNUCTORT DIRTS         [P]           S. NUCTORT CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUGGED.         [P]           S. NEPERTOR OF WELLING OF STRUCTURAL STEEL         [P]           A. FITLEY OF HUELT AND CROOP WELDS         [P]           C. CONTOCID MEDING OF STRUCTURAL STEEL         [P]           C. CONTOL AND FINISH OF ACCESS HOLES         [P]           C. CONTOL AND FINISH OF ACCESS HOLES         [P]           C. CONTOL AND FINISH OF ACCESS HOLES         [P]           C. CONTORMACE WITH WE AND RELDING TECHNOLES         [P]           F. ACCEPTANCE ENVIRONMENT LCONDITIONS (WIND SPEED, PRECIPITATION AND TEMPERATURE)         [P]           G. VERTY THE FOLLOWING FAIL SPECTOR PUBLIES SIZE LENGTH AND LCONDITION (VINID SPEED)         [P]           G. VERTY THE FOLLOWING FAIL SPECTOR PUBLIES SIZE LENGTH AND LCONDIN VISUAL ACCEPTANCE CRITERIA, ACC STRUKES, WEB AAREA WEILDS REFE OF CRACKS, BACKING REMOVED AND WED TABS REMOVED (JE REQUIRED);         [P]           R. INDELEFASS THELET WELDS SIZE AND ONT DETAILS FOR COMPLIANCE:         [P]         [P]           A. BETALE SUCIALS BRACING AND STEPENNE         [P]	3.		[P]	3.
VERTY CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUGGED.         IPT           VERTY CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUGGED.         IPT           A. FITAUP OF FILLET AND GROOVE WILDS         IPT           A. ENTAUP OF FILLET AND GROOVE WILD MATERIALS         IPT           B. CONTROLL & HANDING OVER TRUMPHAL DRAWN TRUES         IPT           B. CONTROLL & HANDING OVER TRUMPHAL DRAWN TRUES         IPT           C. NORROUCH MANNE WILD SARED TACK WILDS         IPT           B. CONTROLL & HANDING OVER TRUMPHAL CONDITIONS (WIND SPEED, PRECIPITATION AND TEMPERATURE)         IPT           F. CONTROLL & HANDING OVER TRUMPHAL DRAWN TRUES SHEED, PRECIPITATION AND TEMPERATURE)         IPT           G. VERTY THE FOLLOWING FOR ALL SPECIFIED WELDS: SIZE, LENTH AND LOCATION VISUAL ACCEPTANCE CITTERIAL AND CONTROL OF STREET, PRAWE AND JOINT DETAILS AT EACH CONNECTION         IPT           H. SINGLEY AND WELDS RESONAND SIMPHENING         IPT         I           C. APPLICATION OF STREET, PRAWE AND JOINT DETAILS AT EACH CONNECTION         IPT         I           C. APPLICATION OF STREET, PRAWE AND JOINT DETAILS AT EACH CONNECTION         IPT         I           MEDINE DREVE CONCORD AND WELDING INSPECTOR ONLIFICATION RECORDS (PDRS) FOR WASS THAT ARE NOT PRECOMMENTING TO THE APPLICATION RECORDS (PDRS) FOR WASS THAT ARE NOT TRUCTURAL STREET, SHALL BE SUBMITTED         IPT           C. MERPECTION OF REVERE AND APPROVALL         IPT         IPT<	4.			<b>-  </b>
Inspection or wellow or 5 multi-flag         IPI           A. Fitzy OF FILET AND GROVE Wellow         IPI           C. CONFIGURATION AND FINISH OF ACCESS HOLES         IPI           D. No WELDING OVER CRACED TACK WELLOW         IPI           C. CONFIGURATION AND FINISH OF ACCESS HOLES         IPI           F. CONFIGURATION AND FINISH OF ACCESS HOLES         IPI           G. VERTY THE FOLLOWING FREE OF CRACKS, BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED); REPAR ACTIVITES         IPI           N. REPERTION OF STELE FRAME AND LOWING TECHNIQUES         IPI         ITI           N. REPERTION OF STELE FRAME AND LOWING TECHNIQUES         IPI         ITI           N. REPERTION OF STELE FRAME AND LOWING TECHNIQUES         IPI         ITI           B. MEMBER LOWATION MEDION AND WELD AND REPERTION AND WELD STARS TERMONICHARLING         IPI         ITI           B. MEMBER LOWATION MEDION INSPECTOR OMPLIANCE:         INTI         INTI         INTICKS           A. WELDING INSPECTION AND WELDING INSPECTOR OLALIFICATION REMOVED AND CONTINUITY RECORDS, AS APPLICABLE SHALLE BOUNT TECHNIQUES         IPI         ITI			[P]	
A. ET-UP-OF FILLET AND GROOVE WELDS         [P]           B. CORFUGATION AND FINISHOF ACCESS HOLES         [P]           C. CONTROL & HANDLING OF WELD MATERNA S         [P]           D. NOW PUTION OF MELD AND TECHNIQUES         [P]           A. DECADENDING OVER CRAVEN TACK WELDS         [P]           E. ACCEPTABLE ENVIRONMENTAL CONDITIONS (WHID SPEED, PRECIPITATION AND TEMPERATURE)         [P]           F. CONFUNDED OVER CRAVEN TACK WELDS TREE OF CONDITIONS (WHID SPEED, PRECIPITATION AND TEMPERATURE)         [P]           F. CONFUNDACE WITH WARD AND WELDING THE CHANGUES         [P]           A. DERABS FILLET WELDS 4's 5/0"         [P]           B. INSPECTION OF STEEL FRAME AND JOINT DETAILS FOR COMPLIANCE:         [P]           A. DETAILS SUCH AS REACING AND STIFFENING         [P]           C. APPLICATION AND MELDING INSPECTOR OULLIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.           NUELSS         VELDING NEPECTION AND WELDING INSPECTOR OULLIFICATION FOR STRUCTURAL STEEL SHALL BE INAL DE IN ACCORDANCE WITH AWS D1.1.           NUELSS         WELDING INSPECTION AND WELDING INSPECTOR OULLIFICATION FOR STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL DE INFORMANCE QUALIFICATION RECORDS FORE (PORE) FOR WEBS TATA ARE NOT PRECURITED           VELENDING PERONNEL DEREGONANCE QUALIFICATION FOR STRUCTURAL STRUCTURAL STRUCTURAL STRUCTURAL DE INFORMANCE QUALIFICATION RECORDS FORE (PORE) FORE WEBS TARA ARE NOT PRECURITED <td< td=""><td>5</td><td></td><td></td><td>┨┝</td></td<>	5			┨┝
C. CONTECL & HANDLING OF WELD MATERIALS         [P]           D. NO WELDING OVER CASKED TACK WELDS         [P]           E. ACCEPTABLE ENVIRONMENTAL CONDITIONS (WIND SPEED, PRECIPITATION AND TEMPERATURE)         [P]           E. ACCEPTABLE ENVIRONMENTAL CONDITIONS (WIND SPEED, PRECIPITATION AND TEMPERATURE)         [P]           E. ACCEPTABLE ENVIRONMENTAL CONDITIONS (WIND SERIED, PRECIPITATION AND TEMPERATURE)         [P]           G. WERRY THE FOLLOWING FOR ALL SPECIFIED WELDS.         [P]           T. MERSEN, WELDS AREA WELDS FREE OF CARCKS; MACKING REMOVED AND WELD TABS REMOVED (P REQUIRED).         [P]           H. SINGLETASS FILLET WELDS 4-5 518         [P]         [P]           G. INSPECTION OF 5TELE FARME AND JOINT DETALS FOR COMPLIANCE:         [P]         [P]           A. DETALS SUCH AS BRACING AND STEFENING         [P]         [P]           G. APPELICATION OF JOINT DETALS FOR COMPLIANCE:         [P]         [P]           A. DETALS SUCH AS BRACING AND STEFENING         [P]         [P]           NUTS:         A. WELDING INSPECTION NOW WELDING INSPECTOR OUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.           WILDING PROCEOURG SPECIFICATIONS (WPSS), PROCEOURG SOLD SPECIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.           WILDING PROCEOURG SPECIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.           WILDING PROCEOURG SPECIFICATION AND ADMENDAL CONTINUT PRECORDERS, SA S	0.		[P]	1
D. NO WELDING OVER CRACKED TACK WELDS         IPI           E. ACCEPTABLE ENVIRONMENTAL CONDITIONS (WIND SPEED, PRECIPITATION AND TEMPERATURE)         IPI           F. CONFORMANCE WITH WES AND WELDING TECHNIDUSE         IPI           G. VERENT VER FOLLOWING TECHNIDUSE         IPI           G. VERENT VER FOLLOWING TORAL SPEED PREDID WELDS: SIZE LENGTH AND LOCATION VISUAL ACCEPTANCE CHITERIA.         F           G. VERENT VER FOR ALL SPEED INDER SIZE LENGTH AND LOCATION VISUAL ACCEPTANCE CHITERIA.         F           H. SINGLEPASS FILLET WELDS 45 510°         IPI         TIT           B. NEPPECTRO OF STELEL TRANK AND JOINT DETAILS FOR COMPLIANCE:         IPI         T           A. DETAILS SUCH AS BRACING AND STIFFENING         IPI         T           B. MENDER ILCOATIONS         IPI         T           C. NEPECTRON OF STELE LARKEN AND JOINT DETAILS FOR COMPLIANCE:         IPI         T           NOTES:         IPI         IPI         T           C. MEDING IN ARD WELDING INSPECTOR OLALIFICATION FOR STRUCTURAL STELL SHALL BE IN ACCORDANCE WITH ANS D1:1.         IPI         IPI           NUELDING PRESONALL PERFORMANCE QUALIFICATION RECORDS (WORD SPORTS THAT ARE NOT PREDULLARIED.         IPI         IPI           VELDING PRESONALL PERFORMANCE QUALIFICATION RECORDS (WORD SPORTS THAT ARE NOT PREDULLARIED.         IPI         IPI           NUELDING PRESONALL PERFORMANCE QUALIFICATION RECORDS (W		B. CONFIGURATION AND FINISH OF ACCESS HOLES		4.
E.         ACCEPTABLE ENVIRONMENTAL CONDITIONS (WIDD SPEED, PRECIPITATION AND TEMPERATURE)         [P]           F.         CONFORMANCE WITH MORE AND WELDING TECHNIQUES         SEE         [P]         5.           F.         CONFORMANCE WITH MORE AND WELDING STREE, LENGTH AND LOCATION WIGUAL ACCEPTANCE OFTERER: ARR STREES, WIE DARREA WELDS 4/# 5/16"         [P]         7.           I.         SINGLE-PASS FILLELT WELDS 4/# 5/16"         [P]         1.           I.         SINGLE-PASS FILLET WELDS 4/# 5/16"         [P]         1.           A.         DETAILS FOR CONNECTION AND MELDING INSPECTOR CONNECTION         [P]         1.           A.         DETAILS FOR CONNECTION AND MELDING INSPECTOR SINCTOR CONNECTION         [P]         1.           A.         DETAILS FOR CONNECTION AND MELDING INSPECTOR ON CONNECTION INTIVITY RECORDS. S. AS APPLICABLE SHALL BE SUBMITTED TO THE APPROVED AGENCY FOR REVEW AND APPROVAL.         [P]         3.           C.         INSPECTION OF CONNECTION AND APPROVAL.         [P]         3.           SINTE MERSHER INSPECTOR IN DETAILS FOR CONSTRUCTION INTOR CONST AS APPLICABLE SHALL BE SUBMITTED TO THE APPROVE				-   -
F.         CONFORMANCE WITH WES AND WELDING TECHNIQUES         [P]           G.         VERIFY THE FOLLOWING FOR ALL SPECTIEP WELDS SIZE, LENGTH AND LOCATION VISUAL ACCEPTANCE ORTERD;         5           ARC STRIKES, WE K AREA WELDS FREE OF CRACKS; BACKING REMOVED AND WELD TABS REMOVED (# FREQUERD);         P]         TTT           I.         INDECTION OF STEEL FAME AND JOINT DETAILS FOR COMPLIANCE:         1           A.         DETAILS SUCH AS BRACING AND STEFFENING         [P]         TT           C.         APELIATION OF JOINT DETAILS FOR COMPLIANCE:         1         1           A.         DETAILS SUCH AS BRACING AND STEFFENING         [P]         1           C.         APELICATION OF JOINT DETAILS AT EACH CONNECTION         [P]         1           NOTES         A         WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS DI 1.         2           NOTES         A         WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE INTER THE AND PLACEMENT         2           TOTISTALATION OF JOINT DETAILS AT EACH CONNECTION         P[P]         3           TOTISTALATION OF AND WELDING INSPECTOR QUALIFICATION KECORDS (CARS) FOR WIDS STRUAT ARE NOT PRECUDALIFICE         2           VERIFY USING HOLES STRUCTURAL         SUBJECTION ON AND WELDING INSPECTOR QUALIFICATION KECORDAND (CONCR. HORE MEDIATION FARONOT PRECORD AND WELDING INSPECTO				┨┝
ARC STRIKES, WEB K-AREA WELDS FREE OF CRACKS; BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED); [P] 7 H SINGLE-PASS FULLET WELDS 4:= 5/10° A DETALS SUCH AS BRACING AND STIFFENING A DETALS SUCH AS BRACING AND STIFFENING B. MEMBER LOCATIONS (MPSS) FROCEDURE OUTLIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1 1. B. WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1 1. B. WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1 1. B. WELDING INSPECTION OF ADDIT DETALS AT EACH CONNECTION WESS, INCOLEDURE SECURICATIONS (WPSS), ROCEDURE OUTLIFICATION RECORDS, ICOR, ISON THAT ARE NOT TRECOLALIFIC TO THE APPROVED AGENOTY OR REVEW AND AD APPROVAL. C. INSTALLATION OF INDI-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 300 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING INGUSTREENT BOLTS. BINEDIC DUCIDATION SCIENCE AND DIANCERS, SUFFACE CONDITIONS AND SUFFORTS. C. INSTALLATION OF REINFORCING STEEL AND PLACEMENT VERIFY VERIFY CHEEN SUFFACE CONSTRUCTION IIEM I INSPECTION OF ROLOCING STEEL AND PLACEMENT VERIFY VERIFY CHEEN SUFFACE OND DIAMETERS, SUFFACE CONDITIONS AND SUFFORTS. 2 INSPECTION OF ROLOCING SECLA IN QUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS, SPECIE 3 INSPECTION OF ROLOCING SECLA IN QUANTITY, MEREDMENT LENGTH, SPACING AND EDGE DISTANCES. (P] 4 4 PERIFY USE OF PLACEMENT F. LOCATION ADD DIAMETERS, SUFFACE CONDITIONS AND SUFFORTS. 2 INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNICURE PLACE BACH DAY SHULL BE OFTIMED AT THE POINT OF A STRUCT EDGE OF APROVED DESIGN MIXTURE FOR EACH TRUCK LOAD 3 VERIFY USE OF PLACEMENT F. LOCATION SPACING, COVER FE MARCHAR ADD DIAMETERS, SUFFACE ARCS AND SUF				
REPAR ACTIVITIES         Image: Construction of a provide the second provide			נסו	5.
INSPECTION OF STELL FRAME AND JOINT DETAILS FOR COMPLIANCE:			[, ]	7
A         DETAILS SUCH AS BRACING AND STIFFENING         [P]         1           B         MEMBER LOCATIONS         [P]         [P]           C         APPLICATION OF JOINT DETAILS AT EACH CONNECTION         [P]           NOTES         [P]         [P]           A         WELDING PROCEDURE SPECIFICATIONS (WPSS), PROCEDURE QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.           B         WELDING PROCEDURE SPECIFICATIONS (WPSS), PROCEDURE QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.           B.         WELDING PROCEDURE SPECIFICATIONS (WPSS), PROCEDURE QUALIFICATION FECOROS, SA SPILICAELE, SHALL BE SUBMITTED           C         INSTRUCTION OF HIGHTRENTH HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL           JOINTS USING INIGHTSTRENTH HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL           JOINTS USING INIGHTSTRENT HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL           JUNTS USING INIGHTSTRENT HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL           JUNTS USING INIGHTSTRENT HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL           JUNTS USING INIGHTSTRENT HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL           JUNTS USING INIGHTSTRENT HOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 380 & RCSC SPECIFICATION FOR STRUCTURAL			[P]	
B.         MEMBER LOCATIONS         [P]           C.         APPLICATION OF JOINT DETAILS AT EACH CONNECTION         [P]           NOTES:         (C)         APPLICATION OF JOINT DETAILS AT EACH CONNECTION         [P]           NOTES:         (C)         APPLICATION AND WELDING INSPECTOR QUALIFICATION FROM STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH ANS D1.1.         (E)           NUELDING PROCEDURE SPECIFICATIONS (MPS), PROCEDURE QUALIFICATION RECORDS (PORS) FOR WPSS THAT ARE NOT PREQUALIFIED, WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (PORS) FOR WPSS THAT ARE NOT PREQUALIFIED, WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (PORS) AS APPLICABLE, SHALL BE SUBMITTED           TO THE APPROVED AGENCY FOR REVEVE NA DA PAPROVAL.         (E)           C.         INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING MICH.STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING MICH.STRENGTH BOLTS.           JINSPECTION OF CONCRETE CONSTRUCTION         FREQUENCY           VERIFY USE OF PLACING STEEL AND PLACEMENT         [P]         3.           VERIFY USE OF ADACHORS CAST IN CONCRETE         SUPRACE CONSTRUCTIONS AND SUPRACE CONDITIONS AND SUPRACE.         [P]         3.           2         INSPECTION OF ANCHORS CAST IN CONCRETE         SUPRACE CONTROL STRUCKLOAD         [P]         3.           3.         VERIFY USE OF APAPROVED DESIGN INSTRUE FOR EACH TRUCK LOAD <td>6.</td> <td></td> <td>[D]</td> <td>_ 1.</td>	6.		[D]	_ 1.
NOTES       A. WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.				┨╞
A. WELDING INSPECTION ADD WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AND VIEW SD.1.1.       Image: Control of Control of ProceDure Specifications (wPS), PROCEDURE QUALIFICATION RECORDS (PGRS) FOR WESS THAT ARE NOT PREQUALIFIED, WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (WPGRS) AND CONTINUITY RECORDS, AS APPLICABLE, SHALL BE SUBMITTED       2.         C. INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS.       2.         3. INSPECTION OF CONCRETE CONSTRUCTION       FREQUENCY       3.         1. INSPECTION OF CONCRETE CONSTRUCTION       FREQUENCY       3.         1. INSPECTION OF ACTIONS, SEEND DIAMETERS, SUFFACE CONDITIONS AND SUPPORTS.       [P]       3.         2. INSPECTION OF ARCING SCAST IN CONCRETE       VERIFY YEE, FINSH, DIAMETER, LENGTH, QUANTITY, LORGTHMENT LENGTH, SPACING AND EDGE DISTANCES.       [P]       8.         3. VERIFY USE OF ALCING TEMPATE WHERE SPECIFIED.       ITT       4.       PRIOR TO CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       1.         4. TESTING OF STRUCTURAL CONCRETE       INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       3.         5. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       3.       3.         6. INSPECTION OF FORCING KF ROB SINHAE, LOCATION AND DIMENSIONS OF THE CONCRETE MANDURE MALL BE OBTAINED AT THE POINT OF PLA		C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION	[P]	<u> </u>
B. WELDING PROCEDURE SPECIFICATIONS (WPS3), PROCEDURE QUALIFICATION RECORDS (PORDS) FOR WPS3 THAT ARE NOT PREQUALIFED, WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (WPG8) AND CONTINUITY RECORDS, AS APPLICABLE, SHALL BE SUBMITTED TO THE APPROVED AGENCY FOR REVIEW AND APPROVAL.  C. INSTALLATION OF INELATION STRENCTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WTH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENCTH BOLTS.  3 INSPECTION OF CONCRETE CONSTRUCTION  ITEM  FREQUENCY  1. INSPECTION OF REINFORCING STEEL AND PLACEMENT VERIFY GRADE, FINISH, SIZE, BAR QUANTTY, LOCATION, SPACING, COVER, HOCK LENGTHS, SPLICE LENGTHS, SPLICE LOCATIONS, BEND DIAMETERS, SUFFACE CONDITIONS AND SUPPORTS. [P]  3. VERIFY USE OF ANCHORS CAST IN CONCRETE VERIFY ORADE, FINISH, SIZE, BAR QUANTTY, LOCATION, SPACING, COVER, HOCK LENGTHS, VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED. [P]  4. VERIFY VISE, FINISH, DUMATER, L'ENOTH, QUANTTY, EMBEDMENT LENGTH, SPACING AND EUGE DISTANCES. [P]  5. INSPECTION OF ANCHORS CAST IN CONCRETE VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED. [P]  5. INSPECTION OF FORMOVED DESIGN MIXTURE FOR EACH TRUCK LOAD [II] 1. PRIOR TO CONCRETE PLACEMENT, FARICATE SPECIFIED. [I] 3. INSPECTION OF FORMUCED ENDING THE THE TEMPERATURE OF THE CONCRETE. [I] 3. INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED [I] 4. TESTING OF STRUCTURAL CONCRETE [I] 5. INSPECTION OF FORMOVER FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED [I] 4. TESTING OF STRUCTURAL CONCRETE [I] 5. INSPECTION OF FORMOVER FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED [I] 4. TESTING OF STRUCTURAL CONCRETE [I] 5. INSPECTION OF FORMAULT AT A MINIMUM BE TAKEN AS FOLLOWS; (A) AT LEAST ONCE ACX 100 CU, YO, OF CONCRETE STALL BE IN ACCORDANCE WITH AST TALL AT MINIMUM BE TAKEN AS FOLLOWS; (A) AT LEAST ONCE ACX 100 CU, YO, OF CONCRETE STALL BE IN ACCORDANCE WITH AST TALEST SPREADED IN CUT PREASED. [I] 6. INSPECTION OF FORMAULT STRUCTURE TEST				
WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (WPORS) AND CONTINUITY RECORDS, AS APPLICABLE, SHALL BE SUBMITTED       2.         10       THE HAPROVED A GENCY FOR REVEW AND APPROVAL.       C.         10       INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS.       9.         3       INSPECTION OF CONCRETE CONSTRUCTION       9.         11       INSPECTION OF CONCRETE CONSTRUCTION       9.         12       INSPECTION OF CONCRETE CONSTRUCTION       9.         13       INSPECTION OF CONCRETE CONSTRUCTION       9.         14       INSPECTION OF ANCHORS CAST IN CONCRETE       9.         15       INSPECTION OF ANCHORS CAST IN CONCRETE       9.         16       PRIOR TO CONCRETE PLACEMENT FOR EACH TRUCK LOAD       10.         17       PRIOR TO CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 20.5.2       10.         16       INSPECTION OF FORWORK FOR SHARE, LOCATION AND DIMENSIONS OF THE CONCRETE MIXTURE PLACED ADX 'SHALL BE OBTAINED AT THE POINT OF PLACEMENT TOR FORWARE FOR SHARE, LOCATION AND DIMENSIONS OF THE CONCRETE MIXTURE PLACED ADX 'SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL CONCRETE       10.         17       4       TESTING OF STRUCTURAL CONCRETE       4.       TESTING OF TESTING MIXTURE FOR PACH TECHNORE CONCRETE MIXTURE PLACED DACI DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL BE TAKE MA				
C.       INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS.         3       INSPECTION OF CONCRETE CONSTRUCTION         ITEM       FREQUENCY         1.       INSPECTION OF REINFORCING STEEL AND PLACEMENT         VERIEY GRADE, FINISH SIZE, BAR QUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS.       [P]         3.       INSPECTION OF REINFORCING STEEL AND PLACEMENT         VERIEY URADE, FINISH SIZE, BAR QUANTITY, CONTON, SPACING, COVER, HOOK LENGTHS.       [P]         3.       INSPECTION OF ANCHORS CAST IN CONCRETE         VERIEY USE OF PAPROXED DESIGN MUNTURE, FOR EACH TRUCK LOAD       [P]         4.       VERIEY USE OF PAPROXED DESIGN MUNTURE FOR EACH TRUCK LOAD       [[G]         5.       INSPECTION OF CONCRETE FLACEMENT FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND       [[G]       1.         4.       TESTING OF STRUCTURAL CONCRETE       [[C]       1.         5.       INSPECTION OF CONCRETE FLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       3.         6.       INSPECTION OF FORWWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED       [P]       4.         4.       TESTING OF STRUCTURAL CONCRETE       5.       1.       1.       1.         7.       INSPECTION OF CONCRETE PLACEMEN			BE SUBMITTED	2.
3         INSPECTION OF CONCRETE CONSTRUCTION           ITEM         FREQUENCY           1         INSPECTION OF REINFORCING STEEL AND PLACEMENT         [P]           2         INSPECTION OF REINFORCING STEEL AND PLACEMENT         [P]           3         INSPECTION OF REINFORCING STEEL AND PLACEMENT         [P]           4         VERIEY GRADE, FINISH SIZE BAR OUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS, SPLICE LENGTHS, SPLICE LOCATIONS, BEND DIAMETER, SURFACE CONDITIONS AND SUPPORTS.         [P]           2         INSPECTION OF ANCHORS CAST IN CONCRETE VERIEY USE OF PAPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD         [T]           4         VERIEY USE OF APPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD         [C]         1.           4         REIGN TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND         [C]         1.           4         INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE         [C]         3.           5         INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE         [C]         3.           4         TESTING OF STRUCTURAL CONCRETE         [C]         3.           4         TESTING OF STRUCTURAL CONCRETE         [P]         4.           5         SAMPLES FOR PREPARING STRENGT THEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SH	C.	INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR ST	RUCTURAL	
ITEM         FREQUENCY           1.         INSPECTION OF REINFORCING STEEL AND PLACEMENT         [P]         3.           VERIFY GRADE, FINISH, SIZE, BAR QUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS, SPLICE LENGTHS, SPLICE LOCATIONS, BEND DIAMETERS, SURFACE CONDITIONS AND SUPPORTS.         [P]         3.           2.         INSPECTION OF ANCHORS CAST IN CONCRETE         [P]         3.           VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.         [P]         8.           3.         VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.         [P]         8.           3.         VERIFY USE OF APPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD         [T]         1.           4.         PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND         [C]         1.           6.         INSPECTION OF FORNWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE.         [C]         3.           6.         INSPECTION OF FORNWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED         [P]         4.           4         TESTING OF STRUCTURAL CONCRETE         5.         SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT FOR SLAPS ONCE FOR EACH 50.00S OF ONCHACH EXITY OF PLACEMENT FOR SLAPS ONCE WITH AST ONCE FOR EACH 100 CU DO F         A.           A.         SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CON				
1.         INSPECTION OF REINFORCING STEEL AND PLACEMENT VERIFY GRADE, FINISH, SIZE, BAR QUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS, SPLICE LENGTHS, SPLICE LOCATIONS, BEND DUMETERS, SURFACE CONDITIONS AND SUPPORTS.         [P]         3.           2.         INSPECTION OF ANCHORS CAST IN CONCRETE VERIFY TYPE, FINISH, DIAMETER, LENGTH, QUANTITY, EMBEDMENT LENGTH, SPACING AND EDGE DISTANCES.         [P]         8           3.         VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.         [P]         8           3.         VERIFY USE OF APPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD         [II]         1.           4.         PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.         [C]         1.           5.         INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED         [P]         4.           4.         TESTING OF STRUCTURAL CONCRETE         5.         INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5.000 S F. OF SURFACE AREA FOR SLABS OW WALLS. SAMPLING OF CONCRETE MIXTURE, PLACED BACH MAST MULL BEND ACCORDANCE WITH ASTM C132.         NO' A           8.         WHEN REREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, PLACED BACH DAST ME CAND AT ADDIMY IS LEAST ONCE FO	3	INSPECTION OF CONCRETE CONSTRUCTION		┛╞
VERIFY GRADE, FINISH, SIZE, BAR QUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS,         [P]         3.           SPLICE LENGTHS, SPLICE LOCATIONS, BEND DIAMETERS, SURFACE CONDITIONS AND SUPPORTS.         [P]         8.           2.         INSPECTION OF ANCHORS CAST IN CONCRETE         [P]         8.           VERIFY USE OF PLACEMENT, FUNCTIN, EMBEDMENT LENGTH, SPACING AND EDGE DISTANCES.         [P]         8.           3.         VERIFY USE OF AND CONCRETE WHERE SPECIFIED.         [C]         1.           4.         PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND         [C]         1.           AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.         [C]         3.           5.         INSPECTION OF FORWNORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED         [P]         4.           4         TESTING OF STRUCTURAL CONCRETE         5.         5.         5.         5.           A.         SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH ODU CU, U, O, OF CONCRETE MALL BEIN ACCORDANCE WITH ANT MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH CONCRETE MALL BE IN ACCORDANCE WITH ANT ASTM C123.         NOT ACCORDANCE WITH ASTM C123.           8.         WHEN REQUENCY OF TESTING WILL PROVIDE FEWER THAN	11	TEM	FREQUENCY	
SPLICE LENGTHS, SPLICE LOCATIONS, BEND DIAMETERS, SURFACE CONDITIONS AND SUPPORTS.       INSPECTION OF ANCHORS CAST IN CONCRETE         VERIFY USE, OF PLACING TEMPLATE WHERE SPECIFIED.       IPI         3. VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.       IIII         3. VERIFY USE OF PLACEMENT, FABRICATE SPECIFIED.       IIII         AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.       ICI       1.         5. INSPECTION OF CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND       ICI       1.         6. INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED       IPI       4.         4 TESTING OF STRUCTURAL CONCRETE       5.       SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF       NO         A. SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE FOR EACH 100 CU. YD. OF       NO         B. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTOR FOR EACH 5.000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE MIXTURE, TESTING SHALL BE CONDUCTOR FOR MATTERST THAN ENTRE.       NO         A. COORDANCE WITH ASTM C132.       CHERCONDOL OF FORM AT PONID OF PLACEMENT FOR EACH COMPOSITE	1.		[P]	3.
VERIFY TYPE, FINISH, DIAMETER, LENGTH, QUANTITY, EMBEDMENT LENGTH, SPACING AND EDGE DISTANCES.         [P]         8           VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.         ITT         9           3.         VERIFY USE OF APPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD         ITT           4.         PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND         [C]         1.           AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.         [C]         3.           5.         INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2         [C]         3.           6.         INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED         [P]         4.           4         TESTING OF STRUCTURAL CONCRETE         5.         NOT         5.           A. SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5:00 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDRANCE WITH ASTMC 112.         B.           8.         WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.         B.           9.         W			C. 1	
VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.         O           3. VERIFY USE OF APPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD         ITT           4. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND         IC]         1.           AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.         ISSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2         IC]         3.           6. INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED         IP]         4.           7         TESTING OF STRUCTURAL CONCRETE         5.         5.           8. INSPECTION OF FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MEMBER BEING FORMED         IP]         4.           4         TESTING OF STRUCTURAL CONCRETE         5.         5.           A. SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU, YD, OF CONCRETE (C) AT LEAST ONCE FOR EACH 5.000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.         B.           9. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL AT A TAPINIT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NON DE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE, TESTING WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.           0. AIR CONTENT: ASTM	2.		נסו	
4.       PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND       [C]       1.         AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.       [C]       3.         5.       INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       3.         6.       INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED       [P]       4.         7.       TESTING OF STRUCTURAL CONCRETE       5.       5.         8.       SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5.000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.       B.         8.       WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE. TESTING SHALL BE CONCRETE CONSISTENCY APPEARS TO CHANGE.       B.         9.       SLUMPE. ASTM C133. ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.       B.         0.       SLUMPE. ASTM C133. (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.       E. TEMPERATURE: ASTM C133. CONCRETE SAMPLE PAR ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173. (VOLUMETRIC METHOD) FOR LIGHT-			[[]]	8
AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.       [L]       2.         S. INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       3.         6. INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED       [P]       4.         7. TESTING OF STRUCTURAL CONCRETE       5.       [P]       4.         8. SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5.000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.       NOT         8. WHEN REQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.       B.         C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH CONCRETE.       B.       B.         D AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF FACH CONCRETE.       B.         D AIR CONTENT: ASTM C133; (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.       B.         E. TEMPERATURE, SATM C1047; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.       E.         T. TEMPERATURE: ASTM C10647; TEST EACH CO				
5.       INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2       [C]       2.         6.       INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED       [P]       4.         4.       TESTING OF STRUCTURAL CONCRETE       5.       [C]       3.         A.       SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5.000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCODANCE WITH ASTM C172.       B.         B.       WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.       B.         C.       SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.       B.         D       AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.       E.         E.       TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C174; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD.       G.	4.		[C]	
6.       INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED       [P]       4.         4       TESTING OF STRUCTURAL CONCRETE       5.         A.       SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5,000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.       NOT         B.       WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED DATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.       B.         C.       SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.       B.         D.       ALR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.       E.         E.       TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.       F.       COLD WEATHER CURING; ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD.       G.       COMPRESSIVE ASTM C31; CAST AND CURE FOUR (4) 6X12 OCT LINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH C	5.		[C]	
<ul> <li>A. SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5,000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.</li> <li>B. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.</li> <li>C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.</li> <li>D. AIR CONTENT: ASTM C133; ITEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40° F AND BELOW OR 80° F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074, RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C39; TEST ONE (1) 6X12 OR NE (1) 4X8 SPECIMENS AT 7 DAYS TEST TWO (2) 6X12 OR THREE (3) 4X8 SPECIMENS AT 58 DAYS TEST TWO (2) 6X12 OR THRE (3) 4X8 SPECIMENS AT 58 DAYS</li> <li>TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 58 DAYS</li> <li>TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 58 DAYS</li> </ul>	6.	INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	[P]	
<ul> <li>A. SAMPLEZARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAT STALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5,000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.</li> <li>B. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.</li> <li>C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.</li> <li>D AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD. WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C39; TEST ONE (1) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR THREE (3) 4X8 SPECIMENS AT 56 DAYS ITEST DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>	4	TESTING OF STRUCTURAL CONCRETE		5.
<ul> <li>B. CONCRETE (C) AT LEAST ONCE FOR EACH 5,000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.</li> <li>B. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.</li> <li>C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.</li> <li>D. AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C33; TEST ONE (1) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 26 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>	Α.			
<ul> <li>ACCORDANCE WITH ASTM C172.</li> <li>B. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.</li> <li>C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.</li> <li>D. AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C33; TEST ONE (1) 6X12 OR ONE (1) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 56 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>				А.
<ul> <li>SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.</li> <li>C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.</li> <li>D AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C39; TEST ONE (1) 6X12 OR ONE (1) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 56 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>		ACCORDANCE WITH ASTM C172.		В.
<ul> <li>C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.</li> <li>D. AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C39; TEST ONE (1) 6X12 OR ONE (1) 4X8 SPECIMENS AT 28 DAYS TEST TWO (2) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 56 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>	В.		,	
<ul> <li>D AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C39; TEST ONE (1) 6X12 OR ONE (1) 4X8 SPECIMEN AT 7 DAYS TEST TWO (2) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 56 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>	C.		ACH DAY'S POUR	
<ul> <li>ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.</li> <li>E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.</li> <li>F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.</li> <li>G. COMPRESSION TEST SPECIMENS: ASTM C31; CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.</li> <li>H. COMPRESSIVE STRENGTH TESTS: ASTM C39; TEST ONE (1) 6X12 OR ONE (1) 4X8 SPECIMEN AT 7 DAYS TEST TWO (2) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 56 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.</li> </ul>	D	AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCE	RETE OR	
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ISPECTION OF POST-INSTALLED ANCHORS [ANCHOR BOLTS, RODS & REINFORCING STEEL]	
	FREQUENC
ANUFACTURER, TYPE, DIAMETER, LENGTH AND FINISH OF ANCHOR	[C]
AXIMUM SPECIFIED IMPACT WRENCH TORQUE RATING MAINTAINED FOR SCREW ANCHORS	[C]
CCEPTABILITY OF BASE MATERIAL	[C]
ISPECTION DURING ANCHOR INSTALLATION ERIFY DRILLING METHOD, HOLE DIMENSIONS, HOLE CLEANING, ANCHOR AND ADHESIVE PLACEMENT, NCHOR EMBEDMENT, WRENCH TORQUE, EDGE DISTANCES AND SPACING.	[C]
ISPECTION AFTER INSTALLATION OF ATTACHED ASSEMBLY ERIFY ANCHOR LOCATIONS, SPACING, EDGE DISTANCES, AND ANCHOR FLUSH WITH AND PERPENDICULAR O THE RECEIVING SURFACE. VERIFY ANCHOR HEADS HAVE NOT BEEN CUT OFF AND THAT MANUFACTURER'S TAMP MARK IS READABLE AND HAS NOT BEEN DAMAGED OR OBSCURED.	[C]
S: ANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) AND RELEVANT ICC-ES REPORTS SHALL BE USED ALONG WITH ONSTRUCTION DOCUMENTS TO DETERMINE COMPLIANCE. ONTINUOUS INSPECTION OF ALL POST-INSTALLED ANCHORS SHALL BE REQUIRED, REGARDLESS OF WHETHER PERIODIC INS PERMITTED BY THE RELEVANT ICC-ES REPORTS. RIOR TO ANCHOR INSTALLATION, REVIEW AND VERIFY CONTRACTOR'S INSTALLATION PROCEDURE. ERIFY THAT THE FULL CURE TIME AS OUTLINED IN THE GENERAL NOTES HAS ELAPSED PRIOR TO APPLICATION OF TORQUE C O ANCHOR.	PECTION
SPECTION OF MASONRY CONSTRUCTION	
	FREQUENC
ERIFY COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND PPROVED SUBMITTALS	[P]
T THE START OF MASONRY CONSTRUCTION, VERIFY: PROPORTIONS OF SITE-PREPARED MORTAR	וסז
PROPORTIONS OF SITE-PREPARED MORTAR CONSTRUCTION OF MORTAR JOINTS	[P] [P]
LOCATION OF REINFORCEMENT, CONNECTORS AND ANCHORAGES	[P]
RING MASONRY CONSTRUCTION, VERIFY:	
SIZE AND LOCATION OF STRUCTURAL MEMBERS	[P]
TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL	[P]
MEMBERS, FRAMES, OR OTHER CONSTRUCTION SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT AND ANCHORAGES	[P]
PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)	[P]
NOR TO GROUTING, VERIFY:	
GROUT SPACE IS CLEAN	[P]
PLACEMENT OF REINFORCING AND CONNECTORS AND ANCHORAGES PROPORTIONS OF SITE-PREPARED GROUT	[P] [P]
CONSTRUCTION OF MORTAR JOINTS	[']
RIFY GROUT PLACEMENT	[C]
SPECTION OF WOOD CONSTRUCTION	
	FREQUENC
	FREQUENC
OF DIAPHRAGMS: SHEATHING THICKNESS AND GRADE	[P]
NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES	[P]
NAIL DIAMETER AND LENGTH	[P]
NUMBER OF FASTENER LINES	[P]
SPACING BETWEEN FASTENERS IN EACH LINE AND AT PANEL EDGES	[P]
SHEATHING THICKNESS AND GRADE NOMINAL SIZE AND SPACING OF TYPICAL FRAMING MEMBERS AND AT ADJOINING PANEL EDGES	[P] [P]
NAIL DIAMETER AND LENGTH	[P]
NUMBER OF FASTENER LINES	[P]
SPACING BETWEEN FASTENERS IN EACH LINE AND AT PANEL EDGES	[P]
LOCATION, SIZE AND TYPE OF HOLDOWNS	[P]
LING, BOLTING, ANCHORING AND FASTENING OF:	
DRAG STRUTS AND COLLECTORS HOLD-DOWNS	[P] [P]
PECTION OF SOILS	
	FREQUENC
RIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE REQUIRED BEARING CAPACITY	[P]
RIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	[P]
RFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	[P]
RIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF FILL	[P]
IOR TO PLACEMENT OF CONTROLLED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	[P]
S: HE APPROVED PROJECT GEOTECHNICAL REPORT SHALL BE USED ALONG WITH THE CONSTRUCTION DOCUMENTS TO ETERMINE COMPLIANCE. URING FILL PLACEMENT, THE SPECIAL INSPECTOR SHALL DETERMINE THAT PROPER MATERIALS AND PROCEDURES ARE SED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED PROJECT GEOTECHNICAL REPORT	

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MARK	HEADER	# OF TRIMMER STUDS	# OF KING STUDS
H1	(3) 2x10	1	2
(H2)	(3) 11%" LVL	2	3
(H3)	(3) 14" LVL	3	3

MARK	APA RATED SHEATHING	SHEATHING NAILING	SILL PLATE ANCHORS	HOLDOWN	HOLDOWN ANCHOR	HOLDOWN END STUDS/POST
SW1	<sup>15</sup> 52" (EXTERIOR FACE)	8d @ 4" o.c. (PANEL EDGES) 8d @ 12" o.c. (FIELD)	%"ø x 1'-0" (HOOKED) @ 24" o.c. W/ BPS5/8-6 PLATE WASHERS (RE: NOTE 13 BELOW)	HDU5-SDS2.5 W/ (14) ¼" × 2½" SDS SCREWS INTO END STUDS	<b>%"</b> ø SSTB24	(2) 2x6
SW2	7⁄16"	8d @ 4" o.c. (PANEL EDGES) 8d @ 12" o.c. (FIELD)	%"ø × 1'−0" (HOOKED) @ 18" o.c. W/ BPS5/8−6 PLATE WASHERS (RE: NOTE 13 BELOW)	HDU8-SDS2.5 W/ (20) ¼" x 2½" SDS SCREWS INTO END POST	‰"ø x 30" PAB7−30	6x4 POST

6. RE: 6/S1.2 FOR TYPICAL SSTB HOLDOWN ANCHOR DETAIL.

7. RE: 7/S3.1 FOR PAB ANCHOR DETAIL.

 RE: 7/33.1 FOR PAB ANCHOR DETAIL.
 SET HEIGHT OF SSTB ANCHORS USING EMBEDMENT MARK INDICATED TO ACCOMMODATE 2x SILL PLATE.
 SECURE ANCHORS TO FORMWORK AND ADJACENT REINFORCING BARS TO PREVENT ANCHOR FROM BEING MOVED OR MISALIGNED DURING THE CONCRETE POUR.
 INSTALL HOLDOWN HARDWARE AND ANCHORS IN CONFORMANCE WITH ALL SUPPLIER'S SPECIFICATIONS AND RECOMMENDATIONS, USING SUPPLIER PROVIDED FASTENERS AND ACCESSORIES.

11. HOLDOWN ANCHORS SHALL BE AS SPECIFIED AND SHALL BE CAST-IN-PLACE. POST-INSTALLED WEDGE OR EPOXIED ANCHOR ALTERNATES ARE NOT ACCEPTABLE. 12. RE: 4/S1.2 FOR ADDITIONAL LATERAL STRAPS TO BE INSTALLED AT CORNERS OF OPENINGS LOCATED WITHIN LENGTH OF SHEARWALLS. 13. RE: 7/S1.2 FOR SILL ANCHOR PLATE WASHER INSTALLATION AT LATERAL SHEARWALLS. 14. SHEATHING SHALL BE PROVIDED IN 4x8 PANELS AND INSTALLED WITH LONG DIMENSION ORIENTED HORIZONTALLY.



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### REINFORCING BAR LAP SPLICE SCHEDULE 'TOP BAR' SPLICE LENGTH BAR SIZE 'OTHER BAR' SPLICE LENGTH **#**3 27" 21" #4 27" 35" **#**5 34" 44" 40" #6 52" #7 77" 59" #8 86" 66"

NOTES:
1. 'TOP BARS' ARE HORIZONTAL BARS PLACED SUCH THAT 12" OF FRESH CONCRETE IS CAST BELOW THE BAR.
2. ALL BARS THAT ARE NOT 'TOP BARS' ARE 'OTHER BARS'.
3. REFER TO THE CONCRETE MATERIAL AND SPECIFICATION NOTES FOR ADDITIONAL INFORMATION AND REQUIREMENTS

## STEEL BEAM SHEAR TAB CONNECTION SCHEDULE

BEAM SIZE	NUMBER OF ¾"Ø BOLTS	SHEAR TAB THK x LENGTH	FILLET WELD SIZE	
W14	3	∛3″×9"	<i>Y</i> 4."	
W16	4	<b>¾"</b> ×12"	<i>Y</i> 4"	
W18	4	<b>¾"</b> ×12"	14"	
BOLT SPACING = STANDARD HOLES	TO BE PROVIDED IN SUI HOLES TO BE PROVIDED	PPORTED WEB.		

OPENINGS LOCATED WITHIN LENGTH OF SHEARWALLS INDICATED ON PLAN.

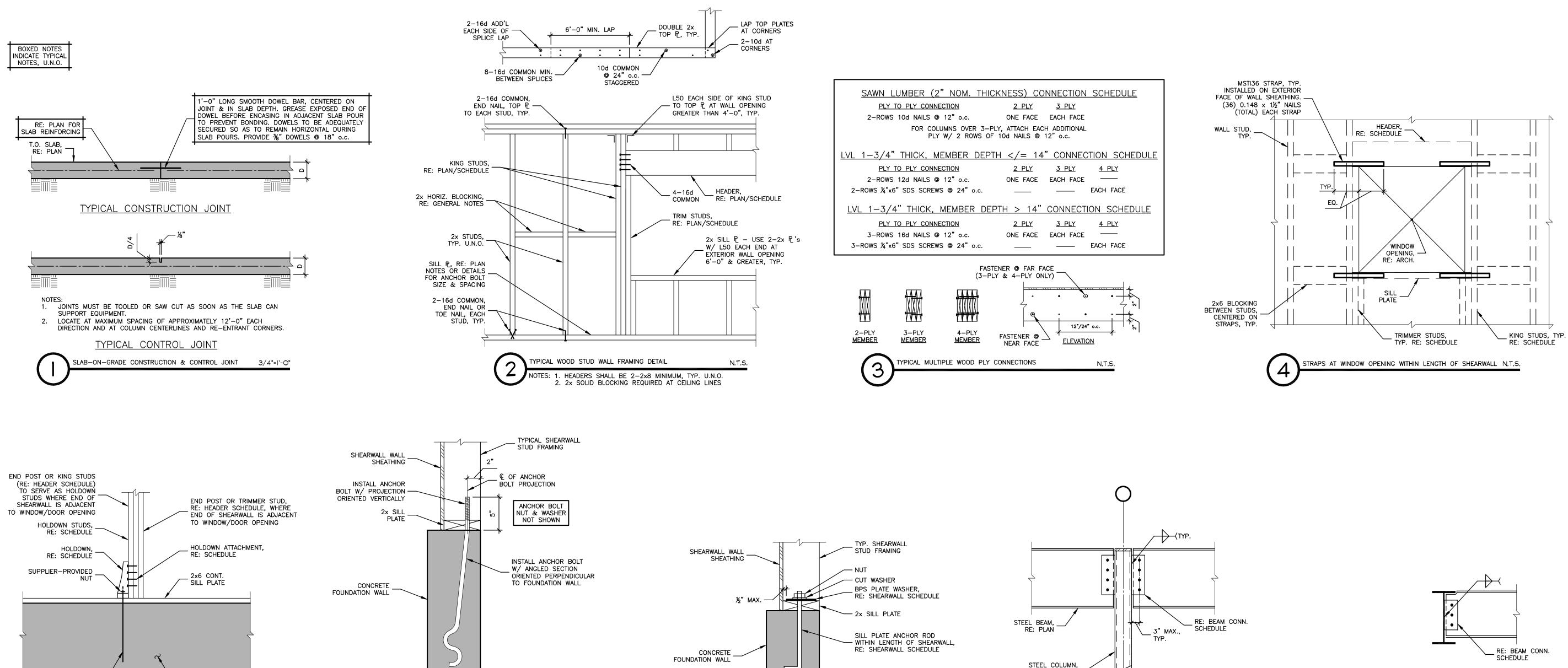
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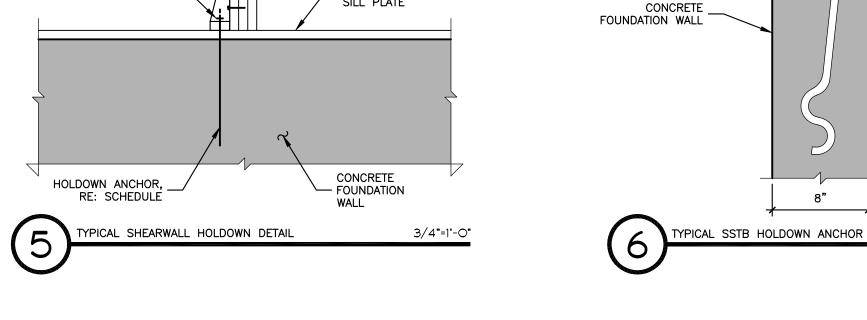
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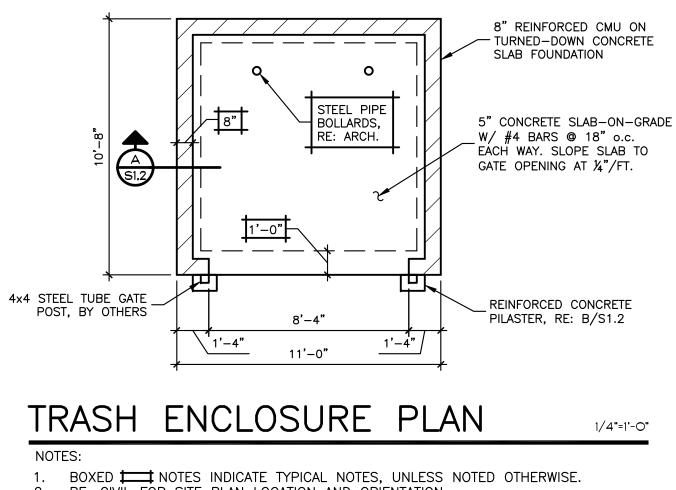
<b>Principal</b> Matt Nichols, P.E. 303.260.9456 matt@nlengineers.c	com
Project Number	25-001
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STRUCTURAL SPECIAL INSPECTIONS & SCHEDULES

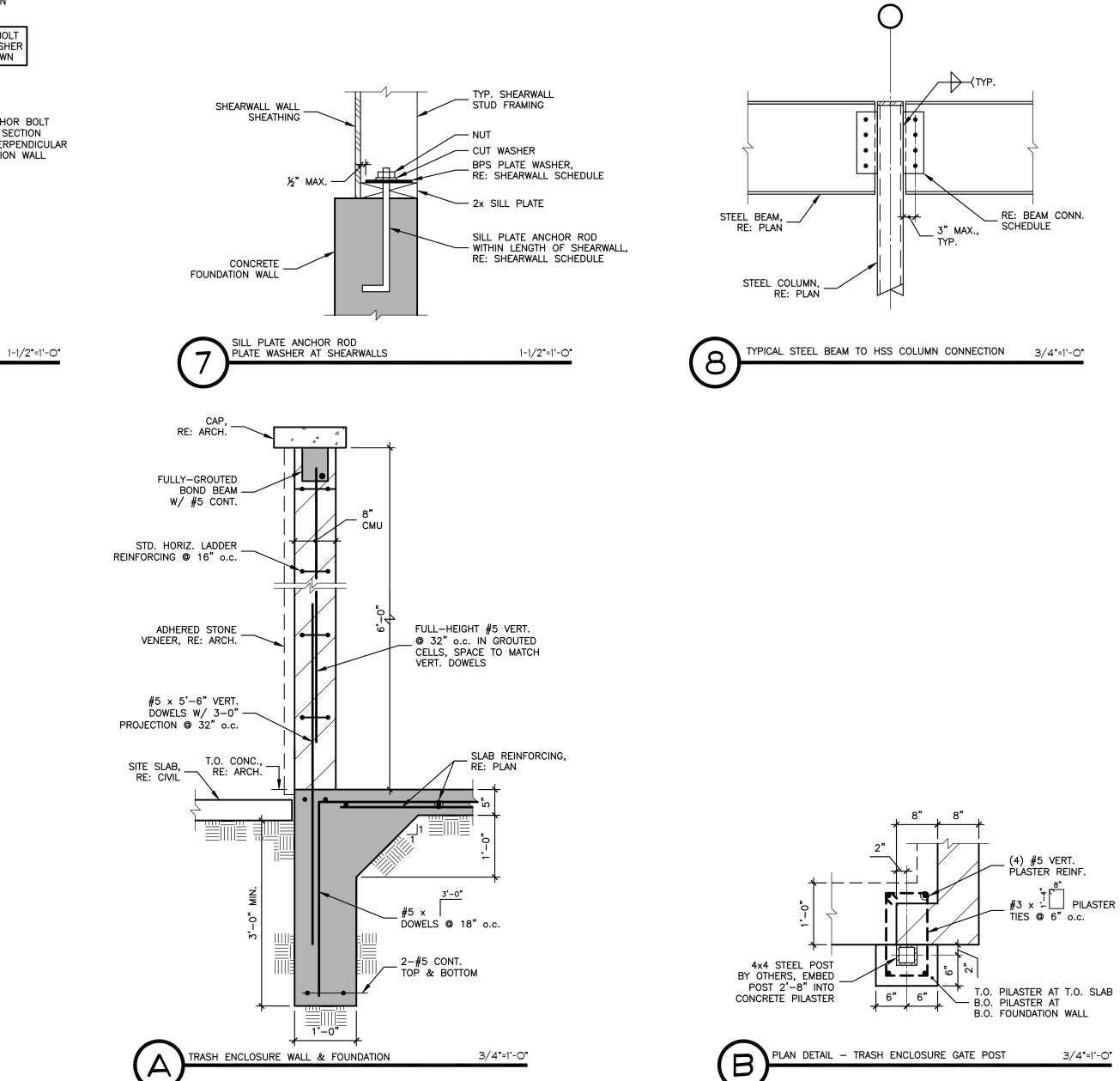
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- RE: CIVIL FOR SITE PLAN LOCATION AND ORIENTATION.
- ENCLOSURE GATE BY OTHERS. 4. SLAB & TURNED-DOWN EDGE TO BE CONSTRUCTED OVER PROPERLY
- PREPARED AND COMPACTED SUBGRADE, RE: PROJECT GEOTECHNICAL REPORT.



TRUCTURAL ENGINEERS nlengineers.com
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<b>Structural Engineer</b> Next Level, Inc. 7186 S. Forest Lane Centennial, CO 80122 303.260.9456
<b>Architect</b> Battista Design Group, P.C. 3650 Wadsworth Blvd. Wheat Ridge, CO 80033

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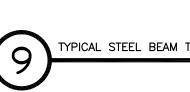
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TYPICAL DETAILS & TRASH ENCLOSURE

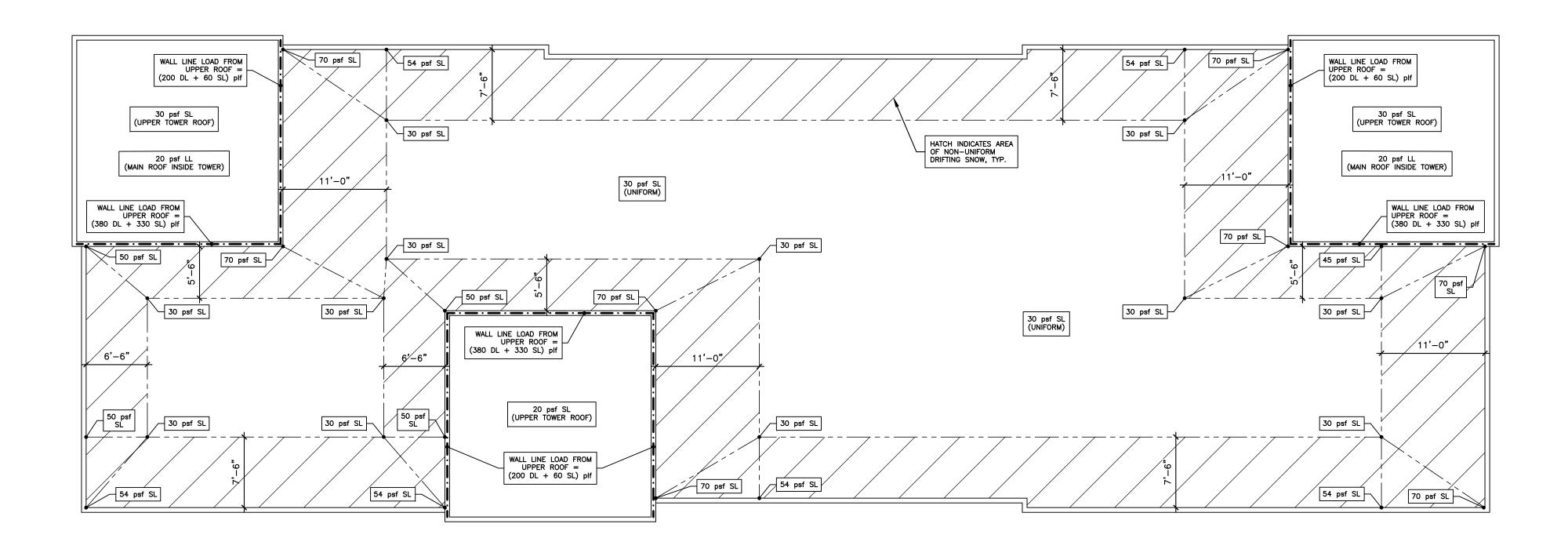
3/4"=1'-0"

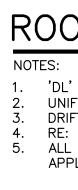


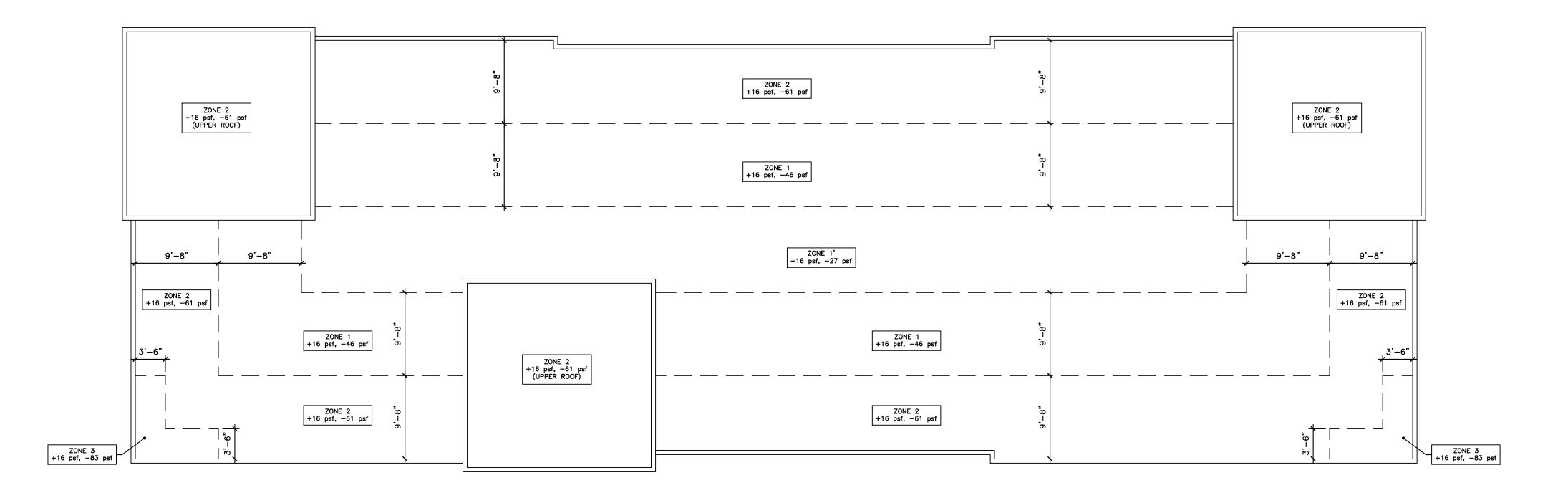
TYPICAL STEEL BEAM TO BEAM CONNECTION

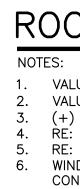
#3 x ÷ □ PILASTER TIES @ 6" o.c.

3/4"=1'-0"









## ROOF GRAVITY DESIGN LOAD DIAGRAM

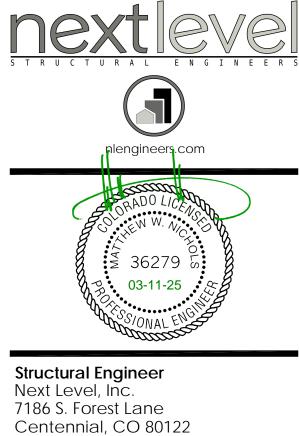
'DL' INDICATES DEAD LOAD. 'LL' INDICATES LIVE LOAD. 'SL' INDICATES SNOW LOAD. UNIFORM SUPERIMPOSED ROOF DL = 15 psf TO BE INCLUDED. DRIFTING SNOW LOADS IN HATCHED AREAS VARY UNIFORMLY BETWEEN VALUES INDICATED. RE: GENERAL NOTES ON SHEET S1.0 FOR ADDITIONAL DESIGN INFORMATION. 5. ALL DESIGN LOADS INDICATED IN THE DRAWINGS SHALL BE USED, CONSIDERING ALL APPLICABLE LOAD COMBINATIONS FROM IBC SECTION 1605.

## ROOF WIND DESIGN LOAD DIAGRAM



N,T,S,

VALUES REPRESENT TOTAL (NOT NET) WIND LOADS.
 VALUES SHOWN ARE ULTIMATE (NOT ASD) LOADS, COMPUTED PER ASCE 7–16.
 (+) VALUES INDICATE DOWNWARD WIND LOADS. (-) VALUES INDICATED UPWARD WIND LOADS.
 RE: GENERAL NOTES ON SHEET S1.0 FOR ADDITIONAL DESIGN INFORMATION.
 RE: DETAIL 1/S4.1 FOR ADDITIONAL VERTICAL PARAPET KICKER WIND LOAD REACTIONS.
 WIND LOADS SHALL BE COMBINED WITH ALL OTHER DESIGN LOADS INDICATED IN THE DRAWINGS, CONSIDERING ALL APPLICABLE LOAD COMBINATIONS FROM IBC SECTION 1605.



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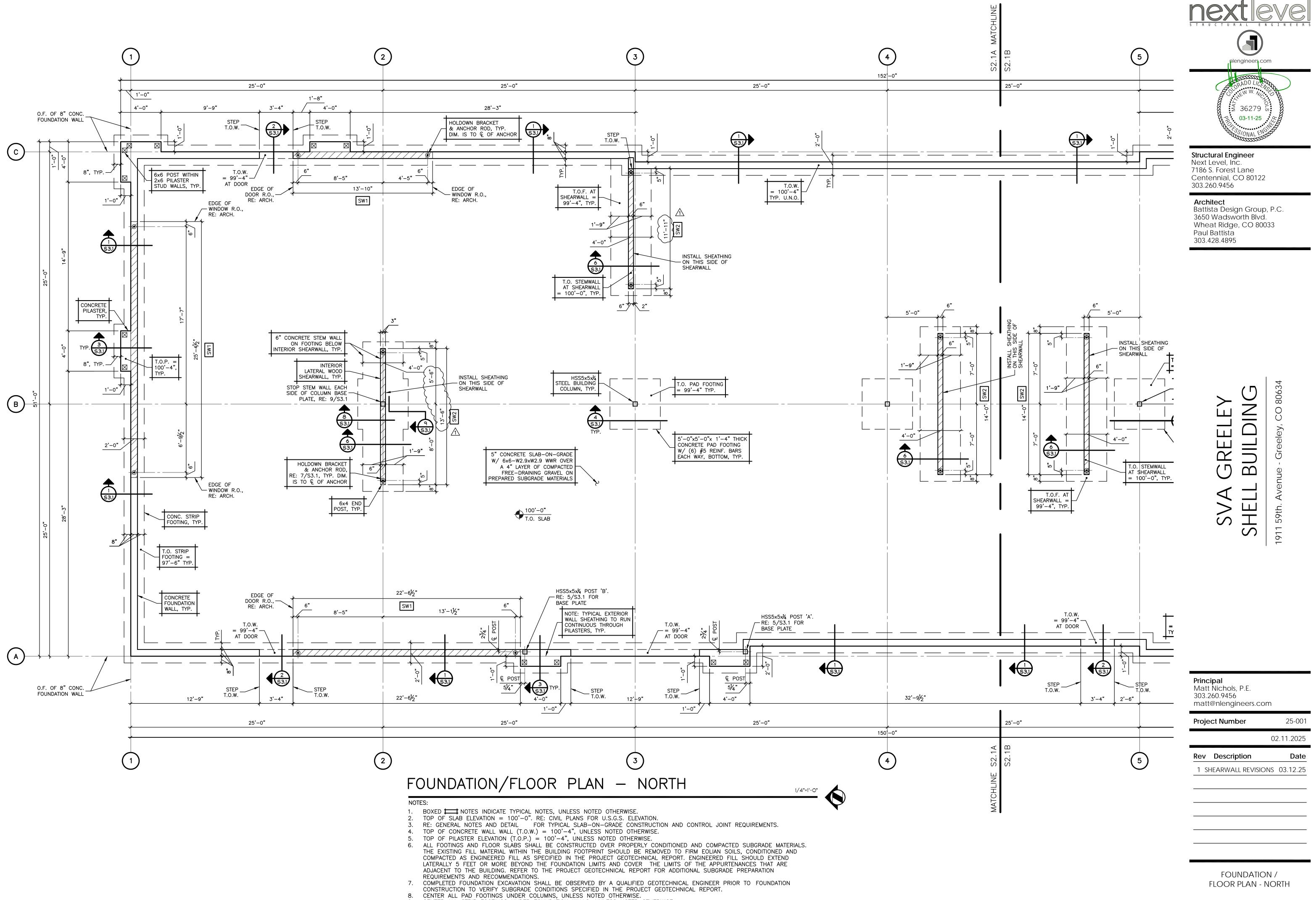
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<b>Principal</b> Matt Nichols, P.E. 303.260.9456 matt@nlengineers.com	
Project Number	25-001
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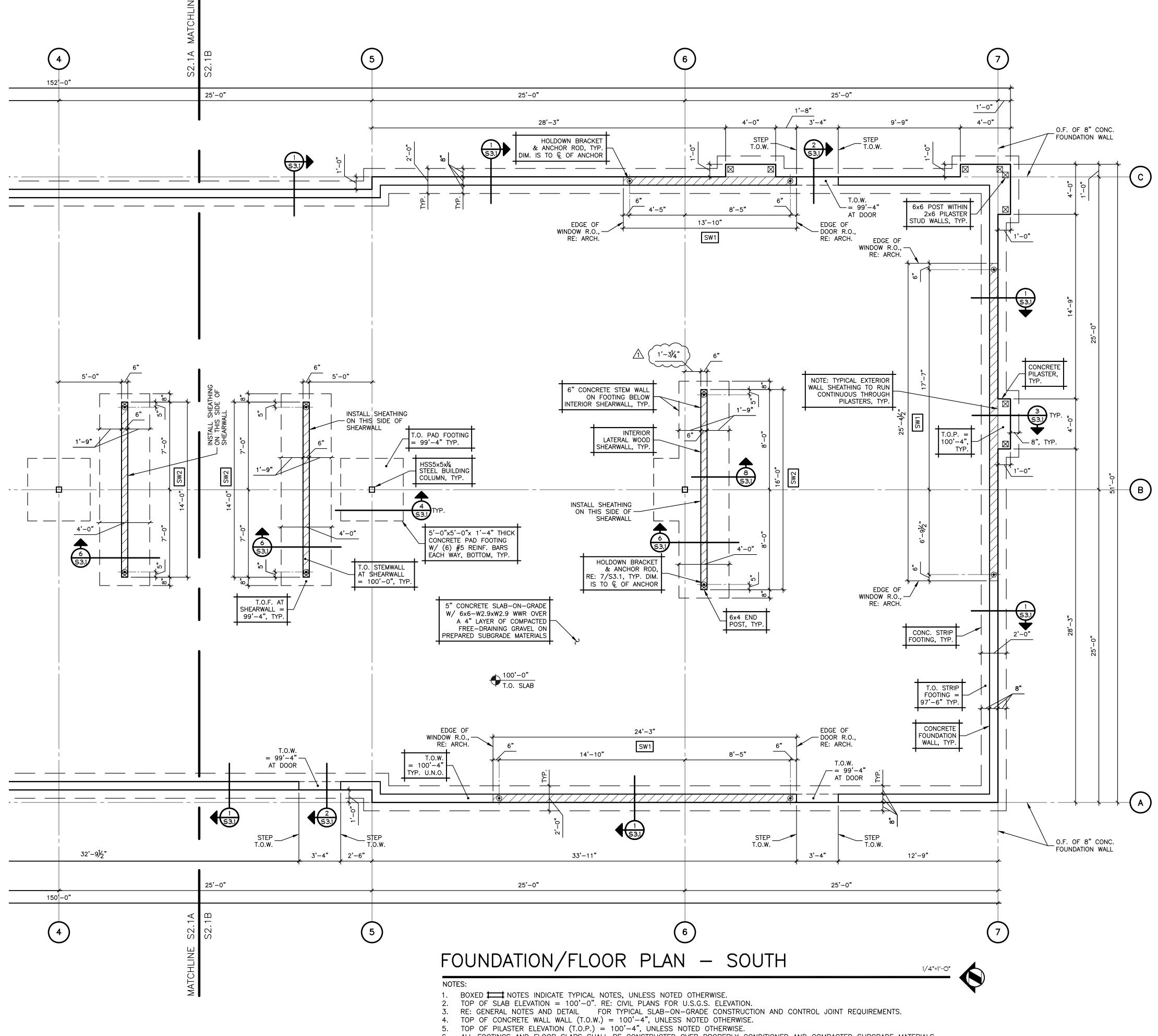
ROOF DESIGN LOAD DIAGRAMS

**S1** 



- 9. CENTER ALL STRIP FOOTINGS UNDER FOUNDATION WALLS, UNLESS NOTED OTHERWISE.
- 10. RE: ARCHITECTURAL AND M/E/P DRAWINGS FOR REQUIRED SLEEVES OR BLOCKOUTS IN FOUNDATION WALLS AND FLOOR SLABS.
- 11. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

S2.1A



ALL FOOTINGS AND FLOOR SLABS SHALL BE CONSTRUCTED OVER PROPERLY CONDITIONED AND COMPACTED SUBGRADE MATERIALS. THE EXISTING FILL MATERIAL WITHIN THE BUILDING FOOTPRINT SHOULD BE REMOVED TO FIRM EOLIAN SOILS, CONDITIONED AND COMPACTED AS ENGINEERED FILL AS SPECIFIED IN THE PROJECT GEOTECHNICAL REPORT. ENGINEERED FILL SHOULD EXTEND LATERALLY 5 FEET OR MORE BEYOND THE FOUNDATION LIMITS AND COVER THE LIMITS OF THE APPURTENANCES THAT ARE ADJACENT TO THE BUILDING. REFER TO THE PROJECT GEOTECHNICAL REPORT FOR ADDITIONAL SUBGRADE PREPARATION

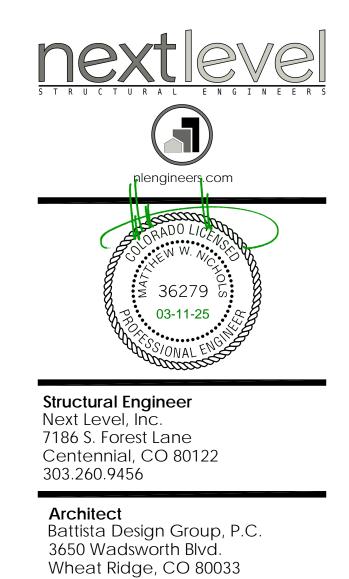
REQUIREMENTS AND RECOMMENDATIONS. 7. COMPLETED FOUNDATION EXCAVATION SHALL BE OBSERVED BY A QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION

CONSTRUCTION TO VERIFY SUBGRADE CONDITIONS SPECIFIED IN THE PROJECT GEOTECHNICAL REPORT.

8. CENTER ALL PAD FOOTINGS UNDER COLUMNS, UNLESS NOTED OTHERWISE.

9. CENTER ALL STRIP FOOTINGS UNDER FOUNDATION WALLS, UNLESS NOTED OTHERWISE.

10. RE: ARCHITECTURAL AND M/E/P DRAWINGS FOR REQUIRED SLEEVES OR BLOCKOUTS IN FOUNDATION WALLS AND FLOOR SLABS. 11. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.



Paul Battista 303.428.4895

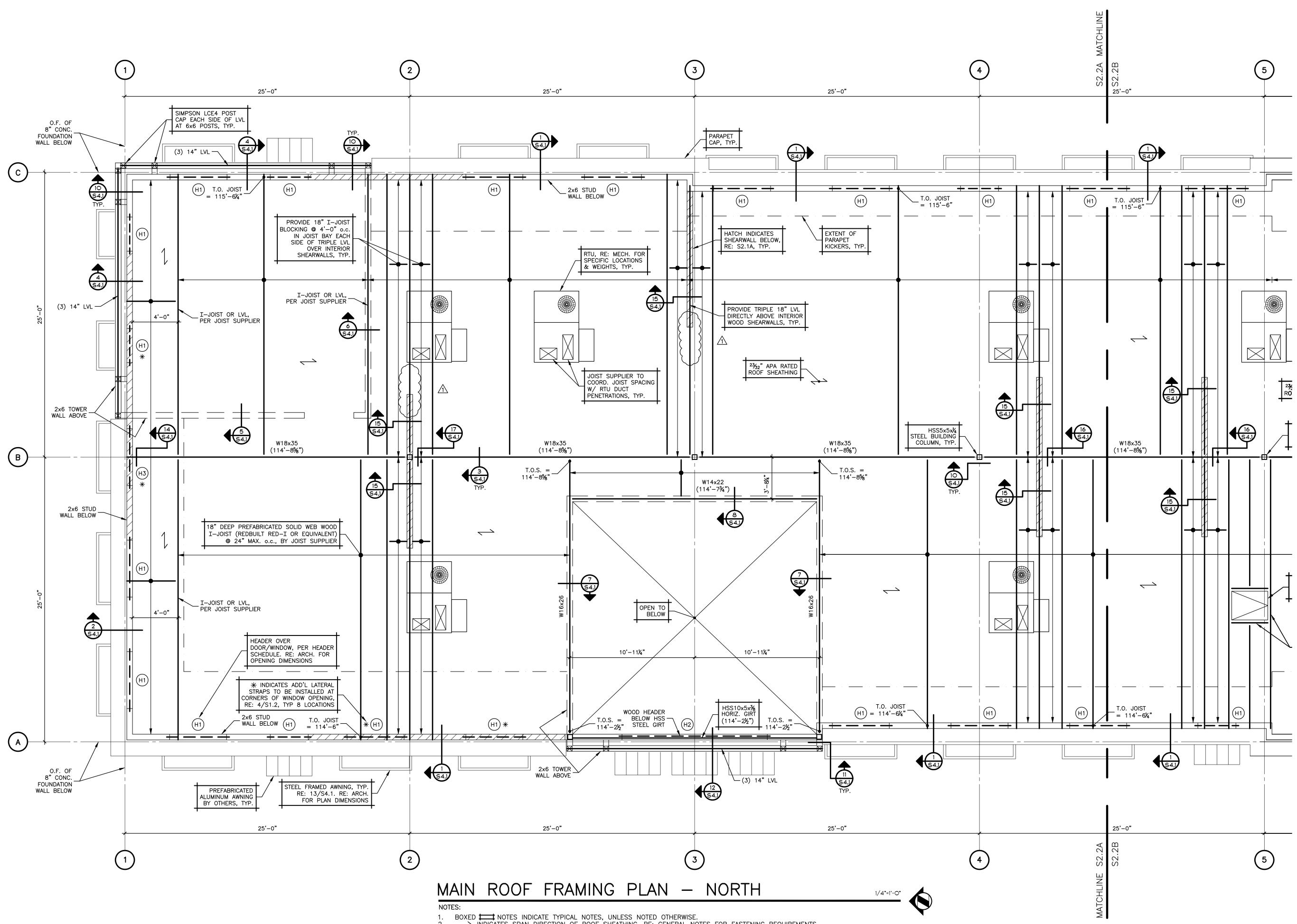
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<b>Principal</b> Matt Nichols, P.E. 303.260.9456 matt@nlengineers.com	
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1 SHEARWALL REVISIONS	03.12.25

FOUNDATION / FLOOR PLAN - SOUTH

S2.1B

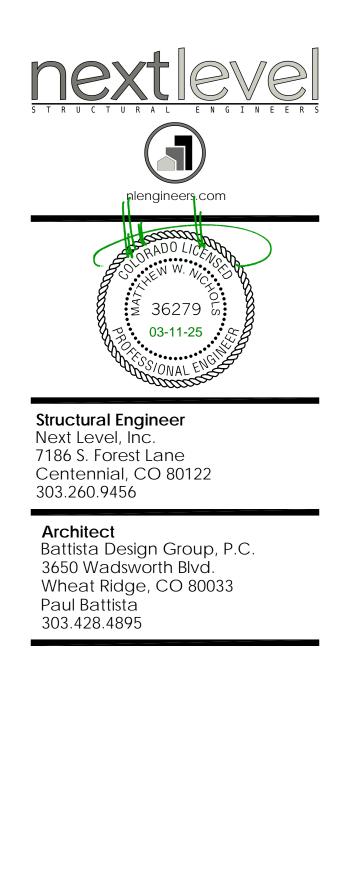


→ INDICATES SPAN DIRECTION OF ROOF SHEATHING. RE: GENERAL NOTES FOR FASTENING REQUIREMENTS.

TOP OF LEVEL STEEL BEAM ELEVATION NOTED THUS: (XXX'-XX"). T.O.S. INDICATES TOP OF STEEL ELEVATION OF SLOPING STEEL BEAM. SLOPE TOP OF BEAMS BETWEEN ELEVATIONS NOTED. COORDINATE SIZE AND LOCATION OF MECHANICAL UNITS AND ROOF PENETRATIONS FOR MECHANICAL DUCTWORK WITH

ARCHITECT, MECHANICAL ENGINEER AND SUPPLIER.

RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

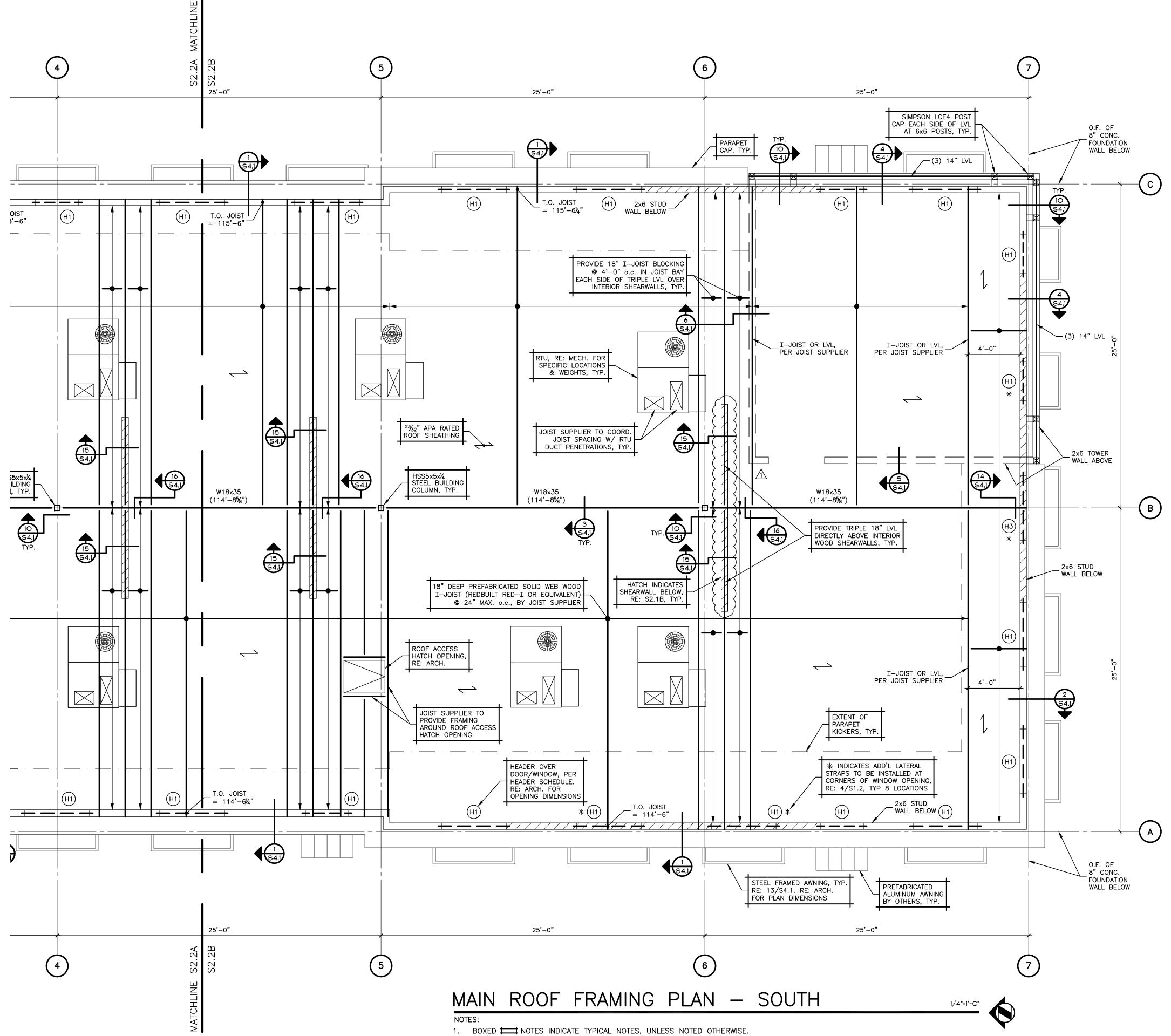




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MAIN ROOF FRAMING PLAN - NORTH

S2.2A

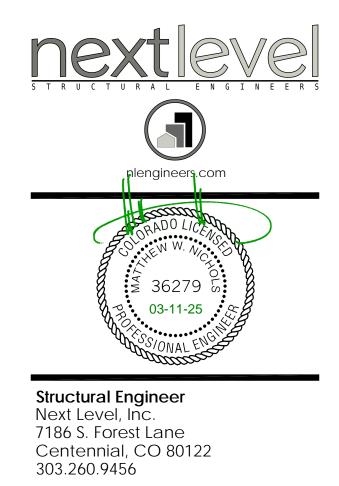


INDICATES SPAN DIRECTION OF ROOF SHEATHING. RE: GENERAL NOTES FOR FASTENING REQUIREMENTS. TOP OF LEVEL STEEL BEAM ELEVATION NOTED THUS: (XXX'-XX").

T.O.S. INDICATES TOP OF STEEL ELEVATION OF SLOPING STEEL BEAM. SLOPE TOP OF BEAMS BETWEEN ELEVATIONS NOTED. COORDINATE SIZE AND LOCATION OF MECHANICAL UNITS AND ROOF PENETRATIONS FOR MECHANICAL DUCTWORK WITH 4. 5.

ARCHITECT, MECHANICAL ENGINEER AND SUPPLIER.

6. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.



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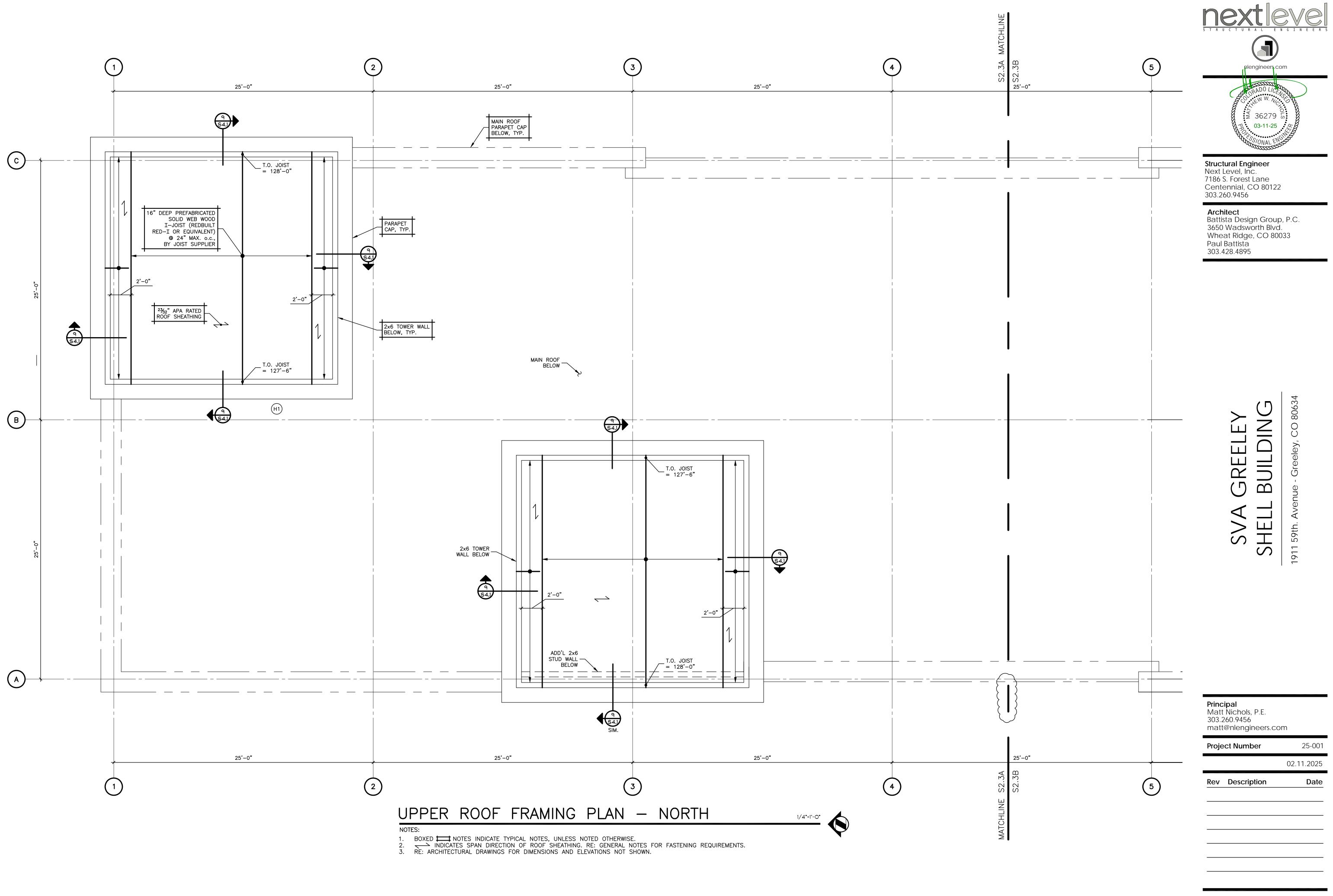
### $\odot$ GREI BUIL $\triangleleft$ SHE S ഹ

91

<b>Principal</b> Matt Nichols, P.E. 303.260.9456 matt@nlengineers.com	
Project Number	25-001
02.11.2025	
Rev Description	Date
1 SHEARWALL REVISION	s 03.12.25

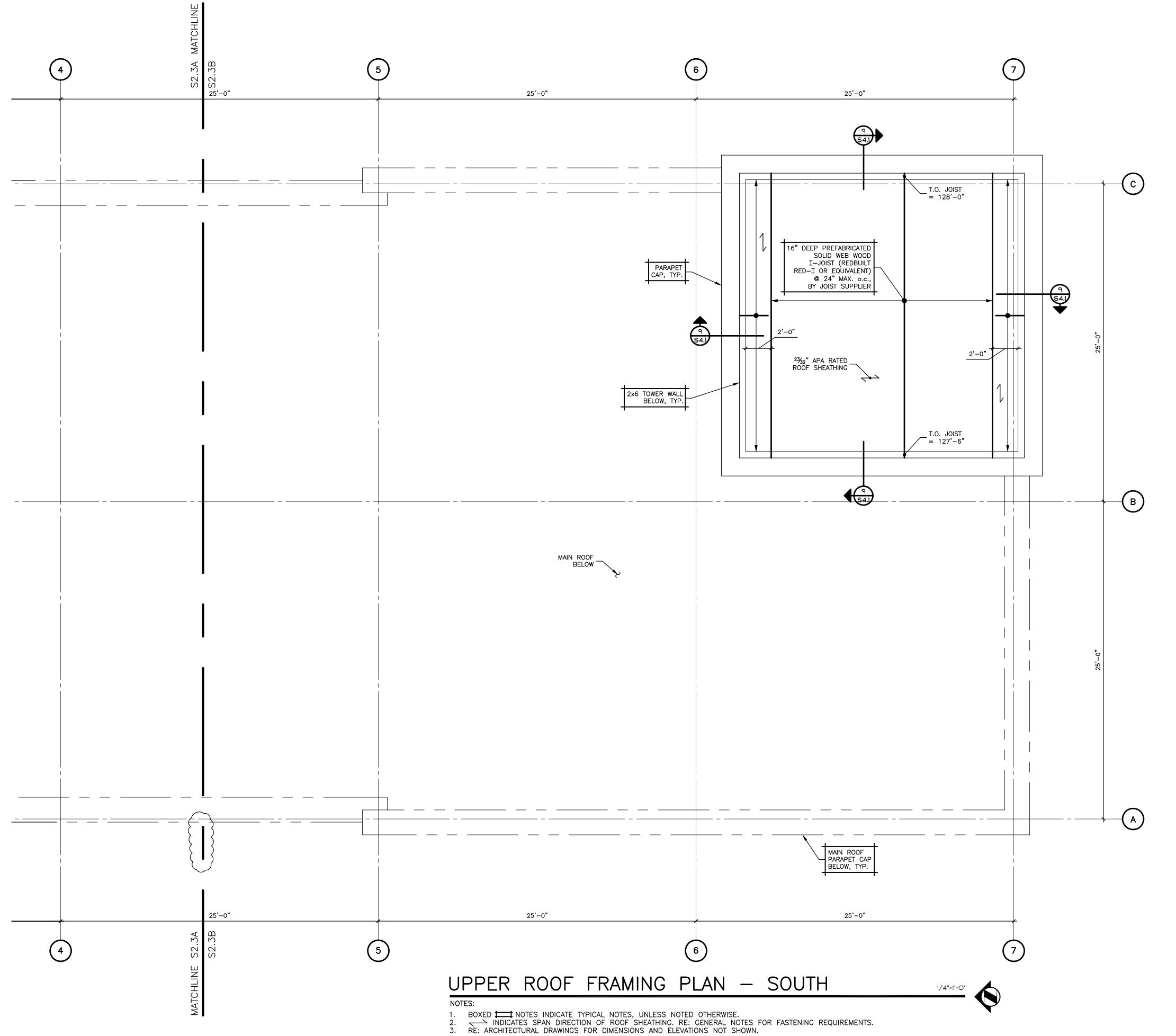
MAIN ROOF FRAMING Plan - South

S2.2B

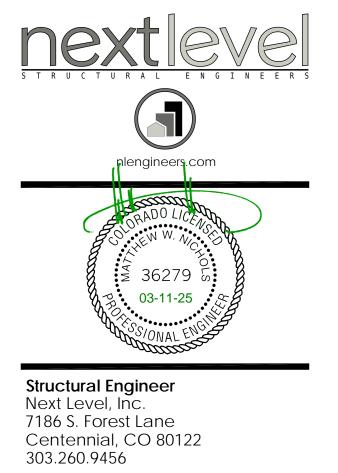


UPPER ROOF FRAMING PLAN - NORTH

S2.3A



3.



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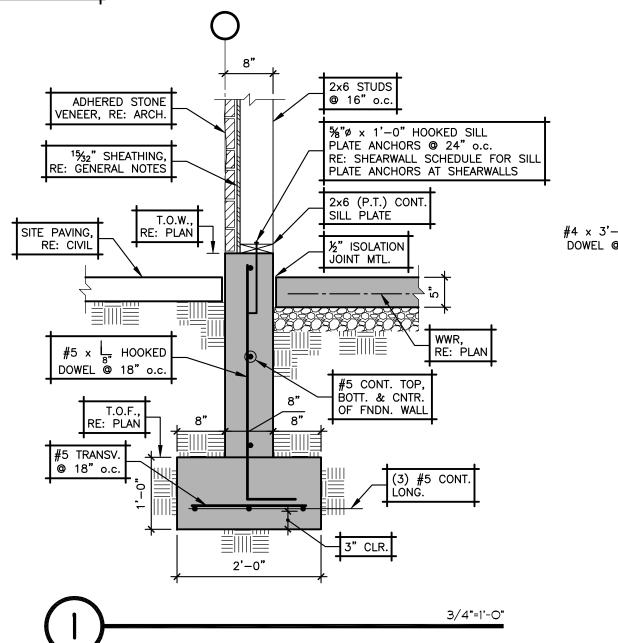
# BUILDING $\succ$ GREELE SHE S

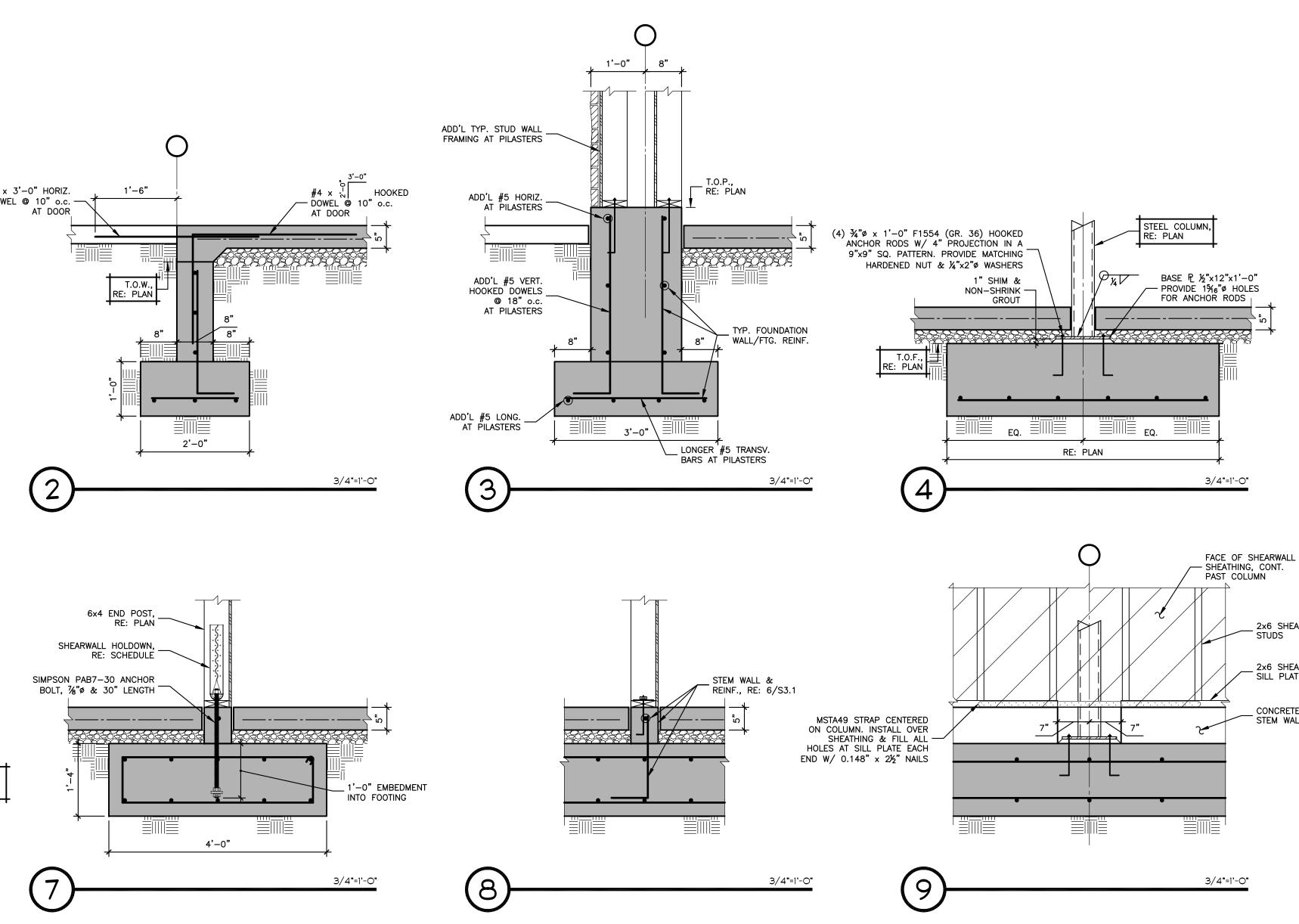
<b>Principal</b> Matt Nichols, P.E. 303.260.9456 matt@nlengineers.com	
Project Number	25-001
	02.11.2025
Rev Description	Date

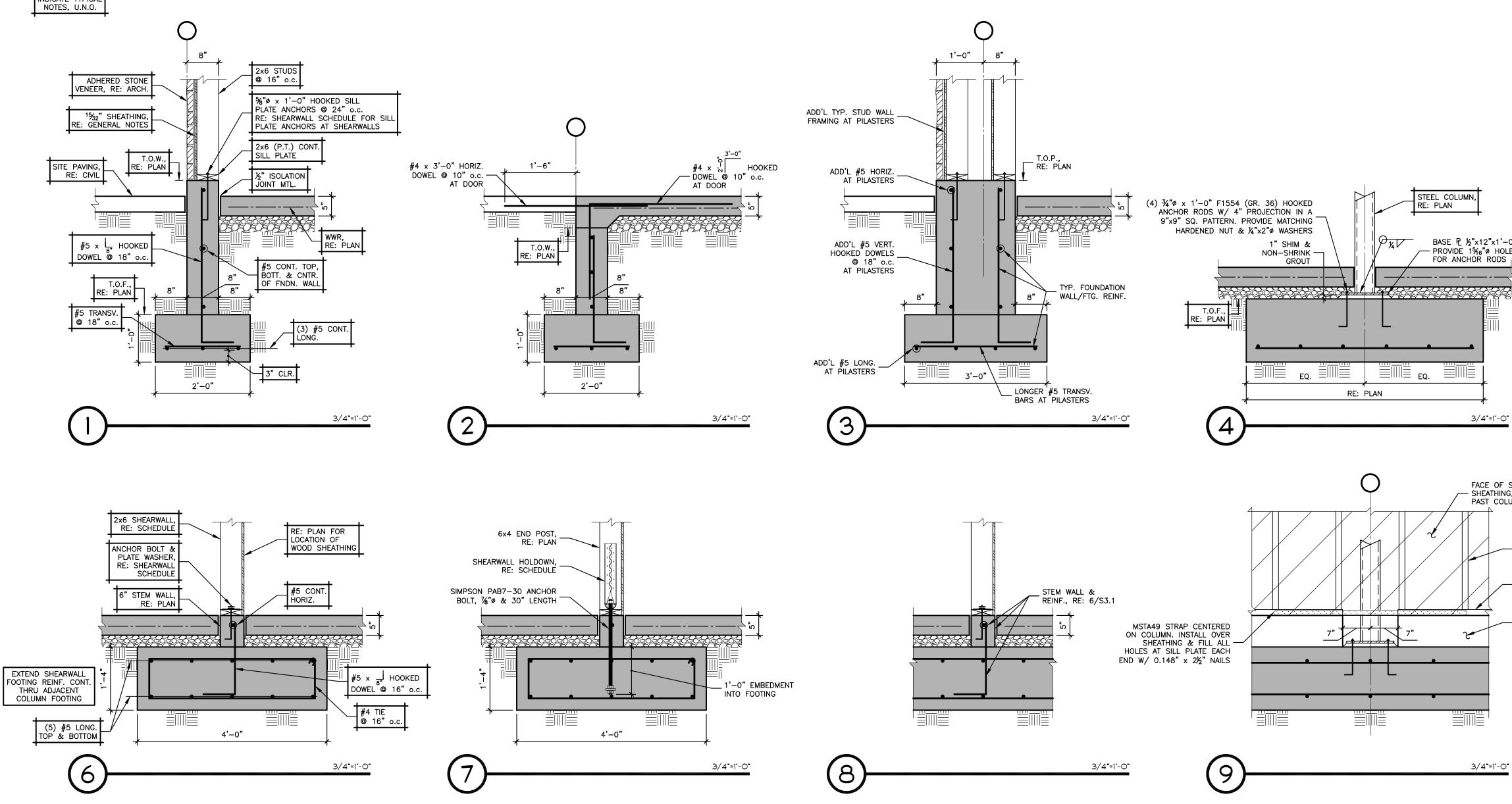
UPPER ROOF FRAMING Plan - South

S2.3B

# BOXED NOTES INDICATE TYPICAL NOTES, U.N.O.









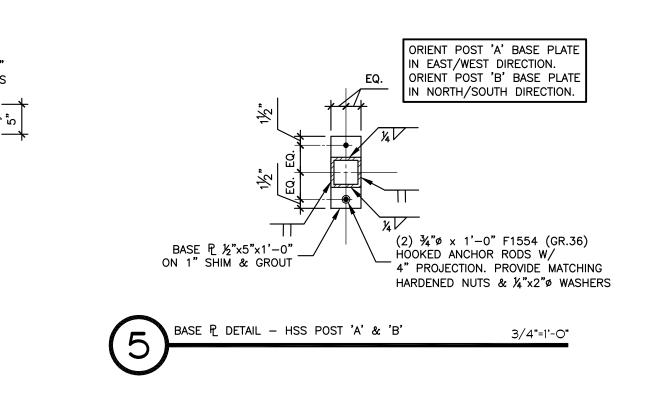
303.260.9456

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STUDS

2x6 SHEARWALL

2x6 SHEARWALL

SILL PLATE

CONCRETE STEM WALL



<b>Principal</b> Matt Nichols, P.E. 303.260.9456 matt@nlengineers.c	om
Project Number	25-001
	02.11.2025
Rev Description	Date
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# Foundation/ Floor details

S3.1

