ABBREVIATIONS

(E)	EXISTING	lb(s)	POUND(S)
/	PER	LLH	LONG LEG HORIZONTAL
@	AT	LLV	LONG LEG VERTICAL
AB	ANCHOR BOLT	LONG	LONGITUDINAL
ABV	ABOVE	LRFD	LOAD RESISTANCE FACTOR DESIGN
ADDL	ADDITIONAL	LT	LIGHT
ADJ	ADJUSTABLE	LT WT	LIGHT WEIGHT
AESS	ARCH. EXPOSED STRUCT. STEEL	LTL	LINTEL
AFF	ABOVE FINISHED FLOOR	LVL	LAMINATED VENEER LUMBER
AHJ	AUTHORITY HAVING JURISDICTION	MAS	MASONRY
AHU	AIR HANDLING UNIT	MAT	MATERIAL(S)
ALT	ALTERNATE	MAX	MAXIMUM
ALUM	ALUMINUM	MECH	MECHANICAL
APPROX	APPROXIMATE	MED	MEDIUM
ARCH	ARCHITECT(URAL)	MEP	MECHANICAL/ELECTRICAL/PLUMBING
ASD	ALLOWABLE STRESS DESIGN	MEZZ	MEZZANINE
BD	BOARD	MFR	MANUFACTURE / MANUFACTURER
BL	BRICK LEDGE	MIN	MINIMUM
BLDG	BUILDING	MISC	MISCELLANEOUS
BLKG	BLOCKING	ML	MICROLLAM
BM	BEAM	MTL	METAL
BO	BOTTOM OF	N	NORTH
BOD	BASIS OF DESIGN	N/S	NEAR SIDE
BOTT	BOTTOM	NO	NUMBER
BRG	BEARING	NOM	NOMINAL
BSMT	BASEMENT	NTS	NOT TO SCALE
BTWN	BETWEEN	oc	ON CENTER
CANT	CANTILEVER	OD	OUTSIDE DIAMETER
CF	COLD-FORMED	OF	OUTSIDE FACE
CIP	CAST-IN-PLACE	OH	OVERHEAD / OPPOSITE HAND
CJ	CONTROL JOINT	OPNG	OPENING
CJP	COMPLETE JOINT PENETRATION	OPP	OPPOSITE
CL	CENTER LINE	OSB	ORIENTED STRAND BOARD
CLG	CEILING	PAF	POWDER ACTUATED FASTENER
CLR	CLEAR	PC	PRECAST
CMU	CONCRETE MASONRY UNIT	PEN	PENETRATION
CNTRD	CENTERED	PERIM	PERIMETER
COL	COLUMN	PERP	PERPENDICULAR
CONC	CONCRETE	PJP	PARTIAL JOINT PENETRATION
CONN	CONNECTION	PL	PLATE
CONST	CONSTRUCTION	PLBG	PLUMBING
CONT	CONTINUOUS	plf	POUNDS PER LINEAL FOOT
COORD	COORDINATE	PLWD	PLYWOOD
D / DL	DEAD LOAD	psf	POUNDS PER SQUARE FOOT
DBL	DOUBLE	psi	POUNDS PER SQUARE INCH
DIA	DIAMETER	PT	POST-TENSIONED/PRESSURE TREATED
DIAG	DIAGONAL	RAD	RADIUS
DIM	DIMENSION	RE	REFERENCE / REFER TO
DIV	DIVISION	REINF	REINFORCED(ING)
DN	DOWN	REQD	REQUIRED
DTL	DETAIL	REV	REVISION
DWG	DRAWING	RO	ROUGH OPENING
DWL	DOWEL	RTU	ROOF TOP UNIT
E	EARTHQUAKE/SEISMIC LOAD / EAST	S	SOUTH
E/F	EACH FACE	S / SL	SNOW LOAD
E/S	EACH SIDE	SA	SUPPLY AIR
EA	EACH	SC	SLIP-CRITICAL
EIFS	EXTERIOR INSUL. FINISH SYSTEM	SCHED	SCHEDULE
EJ	EXPANSION JOINT	SECT	SECTION
ELEC	ELECTRIC(AL)	SF	SQUARE FEET
ELEV	ELEVATION	SHT	SHEET
EMBED	EMBEDMENT	SIM	SIMILAR
ENCL	ENCLOSURE	SOG	SLAB-ON-GRADE
ENGR	ENGINEER	SPEC	SPECIFICATION(S)
EO	EDGE OF	SQ	SQUARE
EOR	ENGINEER OF RECORD	SS	STAINLESS STEEL
EQ	EQUAL	STD	STANDARD
EQUIP	EQUIPMENT	STIFF	STIFFENER
EST	ESTIMATE	STL	STEEL
EXP	EXPANSION	STOR	STORAGE
EXT	EXTERIOR	STRUCT	STRUCTURAL
F/S	FAR SIDE	SYM	SYMMETRICAL
FF	FINISH FLOOR	T&B	TOP AND BOTTOM
FFE	FINISHED FLOOR ELEVATION	T&G	TONGUE AND GROOVE
FIN	FINISH(ED)	T/C	TENSION-CONTROL
FNDN	FOUNDATION	TBD	TO BE DETERMINED
FO	FACE OF	THK	THICK(NESS)
FT	FEET/FOOT	THRU	THROUGH
FTG	FOOTING	TL	TOTAL LOAD
FV	FIELD VERIFY	TO	TOP OF
GA	GAUGE	TOBL	TOP OF BRICK LEDGE
GALV	GALVANIZED	TOC	TOP OF CONCRETE
GC	GENERAL CONTRACTOR	TOF	TOP OF FOOTING
GYP	GYPSUM	TOM	TOP OF MASONRY
GYP BD	GYPSUM BOARD	TOP	TOP OF PILASTER / PIER
HAS	HEADED ANCHOR STUD	TOS	TOP OF STEEL / SLAB
HORZ	HORIZONTAL	TOW	TOP OF WALL
HSS	HOLLOW STRUCTURAL SECTION	TRANSV	TRANSVERSE
HT	HEIGHT	TS	TUBE STEEL
HVAC	HEATING/VENTILATING/AIR CONDITIONING	TYP	TYPICAL
IBC	INTERNATIONAL BUILDING CODE	ULT	ULTIMATE
ID	INSIDE DIAMETER	UNO	UNLESS NOTED OTHERWISE
IF	INSIDE FACE	VERT	VERTICAL
IN	INCHES / INCH	VIF	VERIFY IN FIELD
INCL	INCLUDING	W	WIDE/WIDTH
INSUL	INSULATION / INSULATING	W	WIND LOAD
INT	INTERIOR	W/	WITH
JNT	JOINT	W/C	WATER/CEMENT
JST	JOIST	W/O	WITHOUT
K	KIPS	WD	WOOD
klf	KIPS PER LINEAL FOOT	WF	WIDE-FLANGE
ksf	KIPS PER SQUARE FOOT	WP	WORK POINT
ksi	KIPS PER SQUARE INCH	WT	WEIGHT
L / LL	LIVE LOAD	WWR	WELDED WIRE REINFORCING

SYMBOLS

NORTH ARROW

SECTION DETAIL — DETAIL NUMBER; SHEET REFERENCE

PLAN / GENERAL DETAIL — DETAIL NUMBER; SHEET REFERENCE

ELEVATION DETAIL

SPOT ELEVATION — XXXXX; XXX'–XX"

GRIDLINE - NEW

GRIDLINE - EXISTING

BOXED NOTE INDICATES TYPICAL NOTE — XXXXXXX XXXXXXX

SPAN DIRECTION OF DECK/SHEATHING

MOMENT CONNECTION

REVISION NUMBER

CLOUD REVISION

FOOTING/DRILLED PIER TYPE

COLUMN TYPE

STEP

SLOPE

GRAPHIC SCALE BAR — 0' 2' 4' 8'

MATERIALS

- CAST-IN-PLACE CONCRETE
- PRECAST OR EXISTING CONCRETE
- STEEL
- CONCRETE MASONRY
- BRICK MASONRY
- SOIL SUBGRADE
- GRAVEL FILL
- RIGID INSULATION

SHEET INDEX

NOTE TO CONTRACTOR

READ ALL STRUCTURAL NOTES AND COORDINATE WITH THE STRUCTURAL ENGINEER TO RESOLVE ANY QUESTIONS, DISCREPANCIES, OR CONFLICTS PRIOR TO COMMENCING WITH CONSTRUCTION OPERATIONS.

CONTRACTOR SHALL INFORM AND UPDATE THE STRUCTURAL ENGINEER AS TO THE STATUS AND SCHEDULE OF CONSTRUCTION, AND SHALL COORDINATE WITH THE STRUCTURAL ENGINEER TO SCHEDULE PERIODIC SITE VISITS TO OBSERVE COMPLETED AND ONGOING PORTION OF THE WORK.

STRUCTURAL ENGINEER'S APPROVAL OF ALL REQUIRED SUBMITTALS INDICATED IN THE GENERAL NOTES SHALL BE PROCURED PRIOR TO FABRICATION OR CONSTRUCTION OF EACH APPLICABLE PORTION OF THE WORK.

COORDINATE ALL REQUIRED STRUCTURAL SPECIAL INSPECTIONS AND TESTS WITH THE INSPECTION AGENCY ENGAGED BY THE OWNER.

DESIGN CRITERIA

1. **CODES & STANDARDS:**
   - A. INTERNATIONAL BUILDING CODE 2021 EDITION
   - B. GREELEY, COLORADO CODE AMENDMENTS
   - C. ASCE/SEI 7-16 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES
   - D. ACI 318-19 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE
   - E. ANSI/AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS
   - F. ANSI/AWC NDS-2018 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
   - G. TMS 402/602-2016 BUILDING CODE AND SPECIFICATIONS FOR MASONRY STRUCTURES
2. **GRAVITY LOADS USED IN DESIGN:**

A.	ROOF SNOW LOAD	30 psf + DRIFTING REQUIREMENTS
B.	SUPERIMPOSED ROOF DEAD LOAD	15 psf
C.	FLOOR LIVE LOAD	50 psf
D.	OFFICE PARTITION LIVE LOAD	15 psf
E.	LOBBY, 1ST FLOOR CORRIDOR LIVE LOAD	100 psf
F.	MECHANICAL EQUIPMENT LOADS	RE: MECHANICAL DRAWINGS

   - G. SNOW LOAD DESIGN CRITERIA
     1. GROUND SNOW LOAD, Pg = 30 psf
     2. FLAT ROOF SNOW LOAD, Pf = 21 psf
     3. SNOW EXPOSURE FACTOR, Ce = 1.0
     4. SNOW LOAD IMPORTANCE FACTOR, Is = 1.0
     5. THERMAL FACTOR, Ct = 1.0
3. **WIND LOAD DESIGN CRITERIA:**
   - A. BUILDING RISK CATEGORY II
   - B. BASIC DESIGN WIND SPEED = 115 mph
   - C. EXPOSURE CATEGORY C
4. **SEISMIC LOAD DESIGN CRIERIA:**
   - A. BUILDING RISK CATEGORY II
   - B. SEISMIC IMPORTANCE FACTOR, Ie = 1.0
   - C. SITE CLASS D
   - D. SITE SPECTRAL RESPONSE ACCELERATIONS:
     1. Ss = 0.153
     2. S1 = 0.051
     3. SDs = 0.163
     4. SD1 = 0.082
   - E. SEISMIC DESIGN CATEGORY B
   - F. BASIC SEISMIC FORCE RESISTING SYSTEM: WOOD-FRAMED SHEARWALLS WITH WOOD PANELS
   - G. RESPONSE MODIFICATION FACTOR, R = 6.5
   - H. SEISMIC RESPONSE COEFFICIENT, Cs = 0.0251
   - I. ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE
5. **LATERAL FORCE RESISTING SYSTEM DESCRIPTION:**
   - A. LATERAL FORCE RESISTANCE AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED BY VERTICAL WOOD-FRAMED SHEARWALLS.
   - B. THE WOOD ROOF DECKS SERVE AS HORIZONTAL DIAPHRAGMS THAT DISTRIBUTE LATERAL WIND AND SEISMIC FORCES TO THE VERTICAL LATERAL ELEMENTS. THE VERTICAL LATERAL ELEMENTS CARRY APPLIED LATERAL LOADS TO THE BUILDING FOUNDATIONS.
6. **FOUNDATIONS:**
   - A. SOIL DATA WAS TAKEN FROM RECOMMENDATIONS SET FORTH IN PROJECT GEOTECHNICAL REPORT BY NINYO & MOORE DATED OCTOBER 23, 2024 (PROJECT NO. 803044001). REFER TO GEOTECHNICAL REPORT FOR COMPLETE SOILS INFORMATION.
   - B. MAXIMUM TOTAL LOAD SOIL BEARING PRESSURE CAPACITY USED IN DESIGN IS 3,000 psf.

GENERAL NOTES

1. CONTRACTOR RESPONSIBILITY DURING CONSTRUCTION/ERECTION:
   - A. THE STRUCTURE IS DESIGNED TO FUNCTION AS A COMPLETED UNIT, WITH ALL SPECIFIED ELEMENTS AND CONNECTIONS IN PLACE AND FULLY INSTALLED.
   - B. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN, SPECIFICATION, AND IMPLEMENTATION OF ANY AND ALL TEMPORARY BRACING OR SHORING REQUIRED TO ACCOMMODATE THE CONTRACTOR'S MEANS AND METHODS OF CONSTRUCTION AND SEQUENCES OF ERECTION. SUCH BRACING OR SHORING SHALL BE LEFT IN PLACE AS LONG AS MAY BE REQUIRED FOR SAFETY AND UNTIL ALL STRUCTURAL FRAMING AND DIAPHRAGMS ARE IN PLACE WITH CONNECTIONS COMPLETED.
   - C. THE COMPLETED STRUCTURE HAS BEEN DESIGNED ONLY FOR APPLICABLE CODE-PRESCRIBED LOADS ANTICIPATED DURING ITS SERVICE LIFE AS INDICATED HEREIN. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY ADDITIONAL ENGINEERING AND OTHER MEASURES REQUIRED TO ACHIEVE THE CONTRACTOR'S MEANS, METHODS, AND SEQUENCES OF CONSTRUCTION AND TO WITHSTAND ALL TEMPORARY CONSTRUCTION LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING BUT NOT LIMITED TO ERECTION LOADING AND STOCKPILING OF MATERIALS AND EQUIPMENT.
2. ALL DIMENSIONS ON STRUCTURAL DRAWINGS SHALL BE CHECKED AGAINST ARCHITECTURAL.
3. STRUCTURAL DRAWINGS ARE NOT TO BE SCALED TO DETERMINE DIMENSIONAL INFORMATION, TO VERIFY OR COORDINATE ANY INFORMATION PRESENTED OR FOR ANY OTHER PURPOSE.
4. WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, AND GENERAL NOTES, THE MORE STRINGENT REQUIREMENT SHALL GOVERN.
5. STRUCTURAL ENGINEER'S APPROVAL MUST BE SECURED FOR ALL SUBSTITUTIONS.
6. VERIFY ALL OPENINGS THROUGH FLOOR, ROOF AND WALLS WITH MECHANICAL AND ELECTRICAL CONTRACTORS.
7. STRUCTURAL SPECIAL INSPECTIONS SHALL BE PROVIDED AS SPECIFIED AND IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE.
8. THE STRUCTURAL ENGINEER'S PERIODIC OBSERVATIONS OR PRESENCE ON SITE DOES NOT REPLACE OR PRECLUDE THE NEED FOR ANY INDEPENDENT THIRD PARTY STRUCTURAL SPECIAL INSPECTIONS SPECIFIED HEREIN.
9. SIGNIFICANT PERMANENT EQUIPMENT SIZES, WEIGHTS, AND LOCATIONS INDICATED ON THE DRAWINGS ARE AS PROVIDED TO THE STRUCTURAL ENGINEER DURING DESIGN. CHANGES IN SIZES, WEIGHTS, OR LOCATIONS FROM THAT INDICATED MUST BE SUBMITTED IN WRITING FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER. REQUIRED SUPPORTS OR BRACES NOT SHOWN ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE EQUIPMENT SUPPLIER.
10. THE FOLLOWING IS A LIST OF DELEGATED DESIGN AND/OR PERFORMANCE-SPECIFIED ELEMENTS TO BE DESIGNED BY OTHERS AND PRESENTED FOR APPROVAL AS A DEFERRED SUBMITTAL.
    - A. PRE-MANUFACTURED WOOD I-JOIST AND LVL FRAMING ELEMENTS

CONTRACTOR SUBMITTALS

1. GENERAL CONTRACTOR SHALL PROVIDE THE FOLLOWING MATERIAL SPECIFICATION AND SHOP DRAWING SUBMITTALS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH EACH APPLICABLE PORTION OF THE WORK.
2. ITEMS TO BE SUBMITTED AS SPECIFIED IN THE DRAWINGS:
   - A. CONCRETE DESIGN MIXTURES
   - B. CONCRETE STEEL REINFORCING SHOP DRAWINGS
   - C. STRUCTURAL STEEL SHOP DRAWINGS
   - D. MASONRY MORTAR AND GROUT DESIGN MIXTURES
   - E. MASONRY ELEMENT AND ASSEMBLY SPECIFICATIONS
3. DELEGATED DESIGN AND/OR PERFORMANCE-SPECIFIED ELEMENTS TO BE DESIGNED AND PROVIDED BY THE CONTRACTOR:
   [SUBMITTALS ARE TO INCLUDE SUPPORTING CALCULATIONS STAMPED AND SIGNED BY A QUALIFIED STRUCTURAL ENGINEER LICENSED IN THE STATE OF COLORADO. DRAWINGS AND CALCULATIONS ARE TO INCLUDE THE INDICATED ELEMENTS AND THEIR ATTACHMENTS TO THE MAIN BUILDING STRUCTURE.]
   - A. PRE-MANUFACTURED WOOD I-JOIST AND LVL FRAMING ELEMENTS
4. GENERAL CONTRACTOR SHALL REVIEW EACH SUBMITTAL AND CHECK FOR COORDINATION WITH OTHER WORK AND FOR COMPLIANCE WITH THE CONTRACT DOCUMENTS, AND SHALL AFFIX THEIR STAMP INDICATING APPROVAL PRIOR TO SENDING TO THE ARCHITECT AND STRUCTURAL ENGINEER.
5. SUBMITTALS THAT DO NOT BEAR THE CONTRACTOR'S APPROVAL STAMP WILL NOT BE REVIEWED AND WILL BE RETURNED WITHOUT ACTION.DO NOT REPRODUCE CONTRACT DOCUMENTS, COPY STANDARD PRINTED INFORMATION, OR USE ELECTRONIC DRAWING FILES AS THE BASIS FOR SHOP DRAWINGS.
6. GENERAL CONTRACTOR SHALL PROVIDE COPIES OF ALL FINAL, APPROVED, FOR CONSTRUCTION MATERIAL SPECIFICATION AND SHOP DRAWING SUBMITTALS TO THE ARCHITECT AND STRUCTURAL ENGINEER.

MATERIAL SPECIFICATIONS & NOTES

1. **CONCRETE:**
   - A. CONCRETE SHALL BE MADE WITH STONE AGGREGATE (NORMAL-WEIGHT CONCRETE) AND SHALL DEVELOP THE FOLLOWING 28 DAY COMPRESSIVE STRENGTHS (F'C) WITH THE INDICATED DESIGN MIXTURE PROPERTIES:

1.	FOOTINGS	3,500 psi
2.	FOUNDATION WALLS & PILASTERS	3,500 psi ; 0.55 MAX W/C RATIO ; 5% AIR
3.	INTERIOR SLABS ON GRADE	3,500 psi
4.	SITE TRASH ENCLOSURE SLAB & FOUNDATION	4,000 psi ; 0.40 MAX W/C RATIO ; 6% AIR

   - B. RE: CIVIL FOR CONCRETE MIXTURES TO BE USED FOR EXTERIOR SITE SLAB, PAVING, AND FLATWORK, ETC.
   - C. PROPORTIONS OF MATERIALS IN CONCRETE MIXTURES SHALL BE ESTABLISHED TO PROVIDE WORKABILITY AND CONSISTENCY TO PERMIT CONCRETE TO BE PLACED READILY INTO FORMS AND AROUND REINFORCEMENT, UNDER CONDITIONS OF PLACEMENT TO BE EMPLOYED, WITHOUT SEGREGATION OR EXCESSIVE BLEEDING. CONTRACTOR SHALL SELECT AN APPROPRIATE SLUMP, AND ADMIXTURES MAY BE USED AS NEEDED TO OBTAIN ACCEPTABLE RESULTS.
   - D. TYPE I/II PORTLAND CEMENT SHALL BE USED, UNLESS NOTED OTHERWISE.
   - E. FOR CONCRETE MIXTURES USED FOR FLOOR SLABS, THE MINIMUM CEMENTITIOUS CONTENT SHALL BE 540 lbs. PER CUBIC YARD. WHEN FLY ASH IS USED AS A SUPPLEMENTARY CEMENTITIOUS MATERIAL, QUANTITY SHALL NOT BE LESS THAN 15% NOR MORE THAN 25% BY WEIGHT OF TOTAL CEMENTITIOUS MATERIAL.
   - F. FOR CONCRETE PLACED BY PUMPING, THE DESIGN MIXTURE SHALL BE PROPORTIONED TO ENSURE FLOWABILITY TO FACILITATE PUMPING. ENTRAINED AIR MAY BE USED TO FACILITATE PUMPING, SUBJECT TO THE PROVISIONS AND LIMITS INCLUDED HEREIN.
   - G. WHERE AIR CONTENT IS INDICATED ABOVE, PROVIDE AIR ENTRAINING ADMIXTURES. AIR CONTENT VALUE INDICATED INCLUDES BOTH ENTRAINED AND ENTRAPPED AIR, AND MAY BE PROVIDED WITHIN A RANGE OF +/- 1.5%. AIR CONTENT INDICATED IS BASED ON A NOMINAL AGGREGATE SIZE OF ¾". IF ANOTHER AGGREGATE SIZE IS TO BE USED, ADJUST REQUIRED AIR CONTENT PER ACI 318 TABLE 19.3.3.1.
   - H. THE ADDITION OF ENTRAINED AIR IS NOT PERMITTED IN MIXTURES TO BE USED AS FLOOR SLABS, UNLESS THE CONTRACTOR CAN DEMONSTRATE TO THE ARCHITECT THAT SUCH SLAB MIXTURES WITH ENTRAINED AIR WILL PROVIDE AN ACCEPTABLE FINISH WITHOUT BLISTERS.
   - I. VERIFY ALKALINITY OF CONCRETE SLAB SURFACE, SLAB VAPOR TRANSMISSION AND SLAB FLATNESS/LEVELNESS ARE COMPATIBLE WITH FLOOR SYSTEMS AND ADHESIVES PRIOR TO INSTALLING FLOORING.
   - J. NO CHLORIDE ADMIXTURES SHALL BE ADDED TO CONCRETE WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.
   - K. ALL REINFORCING BARS SHALL BE ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE. WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A185 OR ASTM A497.
   - L. CONCRETE PROTECTION FOR REINFORCEMENT (UNLESS NOTED OTHERWISE):
     1. CONCRETE POURED AGAINST EARTH — 3"
     2. CONCRETE POURED IN FORMS BUT EXPOSED TO WEATHER OR EARTH
        - #5 BARS OR SMALLER — 1-1/2"
        - BARS LARGER THAN #5 — 2"
     3. SLABS AND WALLS NOT EXPOSED TO WEATHER OR EARTH — 3/4"
   - M. NO SPLICES OF REINFORCEMENT SHALL BE MADE AND NO WELDING TO REINFORCING SHALL BE PERMITTED EXCEPT AS DETAILED OR AUTHORIZED BY THE STRUCTURAL ENGINEER. REINFORCING LAP SPLICES ARE TO BE A MINIMUM OF 48 BAR DIAMETERS, UNLESS NOTED OTHERWISE. WIRE FABRIC REINFORCEMENT MUST LAP ONE FULL MESH PLUS 2" AT SIDE AND END LAPS, BUT NOT LESS THAN 6", AND SHALL BE WIRED TOGETHER. MAKE ALL BARS CONTINUOUS AT CORNERS OR PROVIDE CORNER BARS OF EQUAL SIZE AND SPACING.
   - N. DETAIL BARS IN ACCORDANCE WITH THE ACI DETAILING MANUAL AND ACI BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.
   - O. PROVIDE ALL ACCESSORIES NECESSARY TO SUPPORT REINFORCING AT POSITIONS SHOWN.
   - P. PLACE 2-#5 BARS (1 EACH FACE) WITH 2'-0" PROJECTION AROUND ALL OPENINGS AND RE-ENTRANT CORNERS IN CONCRETE SLABS AND WALLS, UNLESS NOTED OTHERWISE.
   - Q. CONCRETE WORK SHALL NOT HAVE JOINTS IN A HORIZONTAL PLANE. ALL CONSTRUCTION JOINTS SHALL BE AS DETAILED OR AS APPROVED BY THE STRUCTURAL ENGINEER.
   - R. ALLOW FOR ADDITIONAL CONCRETE THICKNESS TO COMPENSATE FOR STRUCTURAL MEMBER AND FORMWORK DEFLECTIONS.
   - S. SLAB-ON-GRADE CONTROL JOINTS ARE TO BE SPACED A MAXIMUM OF 12'-0" ON CENTER AND ARE TO COINCIDE WITH COLUMN CENTERLINES AND RE-ENTRANT CORNERS.
2. **STRUCTURAL STEEL:**
   - A. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM A36, EXCEPT WIDE FLANGE SHAPES WHICH SHALL CONFORM TO ASTM A992 (GRADE 50), PIPE SECTIONS WHICH SHALL CONFORM TO ASTM A53 (GRADE B), AND HSS SECTIONS WHICH SHALL CONFORM TO ASTM A500 (GRADE C)
   - B. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE AISC STEEL CONSTRUCTION MANUAL, INCLUDING ANSI/AISC 303 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS.
   - C. ALL WELDERS SHALL HAVE EVIDENCE OF PASSING THE AWS STANDARD QUALIFICATION TESTS, AND HAVE A CURRENT CERTIFICATION.
   - D. MINIMUM WELD SIZE SHALL NOT BE LESS THAN 3/16" CONTINUOUS FILLET WELD, UNLESS NOTED OTHERWISE.
   - E. CONNECTIONS MADE WITH HIGH STRENGTH STEEL BOLTS SHALL CONFORM TO THE AISC SPECIFICATION FOR STRUCTURAL JOINTS USING A325 OR A490 BOLTS.
   - F. ANCHOR RODS SHALL CONFORM TO ASTM F1554 GRADE 36 AND ARE TO BE HOOKED, UNLESS NOTED OTHERWISE.
   - G. FABRICATE BEAMS SUCH THAT ROLLING OR FABRICATION INDUCED CAMBER IS UP AFTER ERECTION.
   - H. NON-SHRINK GROUT SHALL COMPLY WITH ASTM C1107 AND ACHIEVE A MINIMUM COMPRESSIVE STRENGTH OF 6,000 psi AT 28 DAYS.
   - I. ALL STEEL OR STEEL CONNECTIONS PERMANENTLY EXPOSED BELOW GRADE SHALL BE COATED WITH A ZINC RICH PAINT OR AN ASPHALTIC MASTIC.
3. **WOOD - GENERAL:**
   - A. ALL SAWN DIMENSION LUMBER FOR STRUCTURAL FRAMING SHALL BE VISUALLY-GRADED, SURFACED DRY HEM FIR, NO.2 GRADE OR BETTER.
   - B. TIMBER CONNECTORS SPECIFIED ARE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY. CONNECTORS BY OTHER MANUFACTURERS MAY BE USED IF THEY HAVE A CURRENT ICC-ES EVALUATION REPORT AND THEIR LOAD CAPACITY IS EQUAL TO OR GREATER THAN THE CONNECTOR SPECIFIED. USE MANUFACTURER'S FURNISHED FASTENERS.
   - C. INSTALL A CONTINUOUS ROW OF HORIZONTAL BLOCKING AT MID-HEIGHT OF SINGLE STORY WALLS OVER 10'-0" TALL.
   - D. GLUE AND NAIL TOGETHER ALL PLIES OF BUILT-UP MEMBERS.
   - E. PROVIDE FULL-HEIGHT 2x KING STUD AT ALL BEAM AND HEADER BEARING LOCATIONS, UNLESS NOTED OTHERWISE.
   - F. SHEATHING FOR ROOFS AND WALLS SHALL BE APA RATED WITH EXPOSURE 1 BOND CLASSIFICATION, AND SHALL CONFORM TO THE PROVISIONS OF APA PRP-108 OR VOLUNTARY PRODUCT STANDARD PS 1-07.
   - G. SHEATHING THICKNESS AND FASTENING REQUIREMENTS SHALL BE AS FOLLOWS:
     1. SHEARWALLS: ALL DESIGNATED SHEARWALLS SHALL HAVE THE SHEATHING THICKNESS AND NAILING SPECIFIED IN THE DRAWINGS. ALL SHEARWALL PANEL EDGES SHALL BE SOLID BLOCKED AT INTERMEDIATE FRAMING MEMBERS.
     2. NON SHEARWALLS: TYPICAL WALLS NOT DESIGNATED AS SHEARWALLS SHALL HAVE ONE LAYER OF 15/32" SHEATHING FASTENED WITH 8D NAILS @ 6" O.C. ALONG PANEL EDGES AND @ 12" O.C. AT INTERMEDIATE FRAMING MEMBERS. NON SHEARWALL PANEL EDGES NEED NOT BE BLOCKED AT INTERMEDIATE FRAMING MEMBERS.
     3. ROOF DIAPHRAGMS: ONE LAYER OF 23/32" SHEATHING FASTENED WITH 10D NAILS SPACED @ 6" O.C. ALONG PANEL EDGES AND @ 12" O.C. AT INTERMEDIATE FRAMING MEMBERS.
   - H. SMOOTH COMMON NAILS SHALL BE USED ON ALL ROOF AND WALL SHEATHING.
   - I. PLACE SHEATHING WITH 8'-0" DIMENSION PERPENDICULAR TO SPAN OF FRAMING MEMBERS AND WITH END JOINTS STAGGERED. ROOF DIAPHRAGMS SHALL NOT BE GLUED.
   - J. FASTEN ALL WOOD MEMBERS PER IBC TABLE 2304.10.1, UNLESS NOTED OTHERWISE.
   - K. LVL BEAMS AND I-JOISTS SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ALL PLATES, BLOCKING, BRIDGING AND OTHER RELATED ITEMS SHALL BE FURNISHED BY THE MANUFACTURER.
4. **PREFABRICATED WOOD ROOF I-JOISTS:**
   - A. SUPPLIER SHALL DESIGN AND PROVIDE JOISTS IN ACCORDANCE WITH THE DESIGN CRITERIA OUTLINED IN THE DRAWINGS. SUPPLIER SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS STAMPED AND SIGNED BY A COLORADO LICENSED PROFESSIONAL ENGINEER FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO DELIVERY TO THE PROJECT SITE.
   - B. RE: DESIGN CRITERIA NOTES THIS SHEET AND SHEET S1.3 FOR APPLICABLE DESIGN LOADS.
   - C. JOISTS SHALL BE DESIGNED TO PROVIDE FOR MAXIMUM VERTICAL DEFLECTIONS AS FOLLOWS:
     1. MAXIMUM TOTAL LOAD DEFLECTION: L/240 OF JOIST SPAN
     2. MAXIMUM SNOW LOAD DEFLECTION: L/360 OF JOIST SPAN
   - D. ALL JOISTS SHALL BE ERECTED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS, INCLUDING CONSIDERATIONS FOR TEMPORARY BRACING.
   - E. ALL REQUIRED BRIDGING AND BLOCKING SHALL BE INSTALLED PRIOR TO INSTALLING SHEATHING.
5. **MASONRY:**
   - A. CONCRETE MASONRY BLOCK UNITS SHALL BE MEDIUM WEIGHT AND SHALL CONFORM TO ASTM C90.
   - B. MORTAR FOR STRUCTURAL WALLS SHALL CONFORM TO ASTM C270.
   - C. GROUT FOR MASONRY SHALL CONFORM TO ASTM C476 AND SHALL DEVELOP A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,000 psi.
   - D. ALL MASONRY ASSEMBLIES SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH (F'M) OF 1,500 psi IN 28 DAYS.
   - E. STANDARD HORIZONTAL STEEL LADDER MESH REINFORCING SHALL BE PROVIDED IN BED JOINTS OF STRUCTURAL MASONRY WALLS, SPACED AT 16" ON CENTER, UNLESS NOTED OTHERWISE.
   - F. SPECIFIED VERTICAL REINFORCING SHALL EXTEND FOR THE FULL HEIGHT OF THE WALL AND SHALL BE GROUTED IN PLACE USING A MAXIMUM OF 5'-0" LIFTS. HIGH LIFT GROUTING MAY BE DONE UP TO A HEIGHT OF 15'-0" AS LONG AS CLEANOUTS ARE PROVIDED AT THE BASE OF EACH GROUTED CELL. ALL CELLS MUST BE CLEANED PRIOR TO GROUTING, AND ALL VERTICAL REINFORCING MUST BE ADEQUATELY SECURED INTO POSITION.
   - G. PROVIDE ONE ADDITIONAL VERTICAL BAR AT THE FOLLOWING LOCATIONS: WALL CORNERS, ENDS OF WALLS, AND EACH SIDE OF OPENINGS.
   - H. PROVIDE HORIZONTAL BOND BEAM REINFORCED WITH A MINIMUM OF 1-#5 CONTINUOUS BARS AT THE TOP OF ALL STRUCTURAL WALLS
6. **POST-INSTALLED ANCHORS:**
   - A. POST-INSTALLED ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH ALL MANUFACTURER'S RECOMMENDATIONS, SPECIFICATIONS AND PRINTED INSTALLATION INSTRUCTIONS (MPII), AND SHALL BE INSTALLED ONLY INTO DRY BASE MATERIALS.
   - B. FOR EXPANSION ANCHORS INSTALLED INTO CONCRETE, THE CONCRETE BASE MATERIAL SHALL REACH THE REQUIRED MINIMUM COMPRESSIVE STRENGTH (F'C) SPECIFIED IN THE DRAWINGS PRIOR TO ANCHOR INSTALLATION.
   - C. FOR ADHESIVE ANCHORS INSTALLED INTO CONCRETE, THE CONCRETE BASE MATERIAL AT THE TIME OF ANCHOR INSTALLATION SHALL HAVE A MINIMUM AGE OF 21 DAYS, A MINIMUM CONCRETE TEMPERATURE OF 50 DEGREES F, AND SHALL REACH THE REQUIRED MINIMUM COMP

STATEMENT OF STRUCTURAL SPECIAL INSPECTIONS & TESTS

1 GENERAL

A. SPECIAL INSPECTIONS AND TESTS SHALL BE PROVIDED IN ACCORDANCE WITH CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE.
B. THE OWNER OR THE OWNER'S AUTHORIZED AGENT, OTHER THAN THE CONTRACTOR, SHALL EMPLOY ONE OR MORE APPROVED AGENCIES TO PERFORM SPECIAL INSPECTIONS AND TESTS DURING CONSTRUCTION ON THE TYPES OF WORK OUTLINED HEREIN.
C. APPROVED AGENCIES SHALL PROVIDE ALL INFORMATION AS NECESSARY FOR THE BUILDING OFFICIAL TO DETERMINE THAT THE AGENCY MEETS OR EXCEEDS THE APPLICABLE CODE-SPECIFIED REQUIREMENTS.
D. PRIOR TO THE START OF CONSTRUCTION, THE APPROVED AGENCIES SHALL PROVIDE WRITTEN DOCUMENTATION TO THE BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER, DEMONSTRATING THE COMPETENCE AND RELEVANT EXPERIENCE OR TRAINING OF THE SPECIAL INSPECTORS WHO WILL PERFORM THE SPECIAL INSPECTIONS AND TESTS DURING CONSTRUCTION.
E. THE CONSTRUCTION OR WORK FOR WHICH SPECIAL INSPECTION OR TESTING IS REQUIRED SHALL REMAIN ACCESSIBLE AND EXPOSED FOR SPECIAL INSPECTION OR TESTING PURPOSES UNTIL COMPLETION OF THE REQUIRED SPECIAL INSPECTIONS OR TESTS.
F. APPROVED AGENCIES SHALL KEEP RECORDS OF SPECIAL INSPECTIONS AND TESTS AND SHALL SUBMIT REPORTS OF SPECIAL INSPECTIONS AND TESTS CONFORMING TO CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE TO THE BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER.
DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO COMPLETION OF THE APPLICABLE PHASE OF THE WORK.
G. SPECIAL INSPECTIONS SHALL BE PROVIDED ON A PERIODIC OR CONTINUOUS BASIS, AS STATED OR INDICATED BELOW.
[P] PERIODIC INSPECTION – PART-TIME OR INTERMITTENT OBSERVATION BY THE SPECIAL INSPECTOR OF WORK BEING PERFORMED. SPECIAL INSPECTOR SHALL BE PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. OBSERVATION OF ALL WORK (100% VISUAL) SHALL BE MADE AT THE COMPLETION OF THE WORK.
[C] CONTINUOUS INSPECTION – FULL-TIME OBSERVATION BY THE SPECIAL INSPECTOR OF WORK BEING PERFORMED. SPECIAL INSPECTOR SHALL BE PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. OBSERVATION OF ALL WORK (100% VISUAL) SHALL BE MADE AT THE COMPLETION OF THE WORK.

2 INSPECTION OF STEEL CONSTRUCTION

	ITEM	FREQUENCY
1.	MATERIAL VERIFICATION OF STRUCTURAL STEEL A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360 B. FOR OTHER STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE CONSTRUCTION DOCUMENTS	[P]
2.	MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, ANCHOR RODS, NUTS AND WASHERS A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	[P]
3.	MATERIAL VERIFICATION OF WELD FILLER METALS A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATIONS IN THE CONSTRUCTION DOCUMENTS	[P]
4.	INSPECTION OF HIGH STRENGTH BOLTING:	
	A. SNUG-TIGHT JOINTS VERIFY CONNECTED MATERIALS HAVE BEEN DRAWN TOGETHER AND PROPERLY SNUGGED.	[P]
5.	INSPECTION OF WELDING OF STRUCTURAL STEEL:	
	A. FIT-UP OF FILLET AND GROOVE WELDS	[P]
	B. CONFIGURATION AND FINISH OF ACCESS HOLES	[P]
	C. CONTROL & HANDLING OF WELD MATERIALS	[P]
	D. NO WELDING OVER CRACKED TACK WELDS	[P]
	E. ACCEPTABLE ENVIRONMENTAL CONDITIONS (WIND SPEED, PRECIPITATION AND TEMPERATURE)	[P]
	F. CONFORMANCE WITH WPS AND WELDING TECHNIQUES	[P]
	G. VERIFY THE FOLLOWING FOR ALL SPECIFIED WELDS: SIZE, LENGTH AND LOCATION; VISUAL ACCEPTANCE CRITERIA; ARC STRIKES; WEB K-AREA WELDS FREE OF CRACKS; BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED); REPAIR ACTIVITIES	[P]
	H. SINGLE-PASS FILLET WELDS </= 5/16"	[P]
6.	INSPECTION OF STEEL FRAME AND JOINT DETAILS FOR COMPLIANCE:	
	A. DETAILS SUCH AS BRACING AND STIFFENING	[P]
	B. MEMBER LOCATIONS	[P]
	C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION	[P]

NOTES:
A. WELDING INSPECTION AND WELDING INSPECTOR QUALIFICATION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AWS D1.1.
B. WELDING PROCEDURE SPECIFICATIONS (WPSS), PROCEDURE QUALIFICATION RECORDS (PQRS) FOR WPSS THAT ARE NOT PREQUALIFIED, WELDING PERSONNEL PERFORMANCE QUALIFICATION RECORDS (WPQRS) AND CONTINUITY RECORDS, AS APPLICABLE, SHALL BE SUBMITTED TO THE APPROVED AGENCY FOR REVIEW AND APPROVAL.
C. INSTALLATION OF HIGH-STRENGTH BOLTS SHALL BE INSPECTED IN ACCORDANCE WITH AISC 360 & RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS.

3 INSPECTION OF CONCRETE CONSTRUCTION

	ITEM	FREQUENCY
1.	INSPECTION OF REINFORCING STEEL AND PLACEMENT VERIFY GRADE, FINISH, SIZE, BAR QUANTITY, LOCATION, SPACING, COVER, HOOK LENGTHS, SPLICE LENGTHS, SPLICE LOCATIONS, BEND DIAMETERS, SURFACE CONDITIONS AND SUPPORTS.	[P]
2.	INSPECTION OF ANCHORS CAST IN CONCRETE VERIFY TYPE, FINISH, DIAMETER, LENGTH, QUANTITY, EMBEDMENT LENGTH, SPACING AND EDGE DISTANCES. VERIFY USE OF PLACING TEMPLATE WHERE SPECIFIED.	[P]
3.	VERIFY USE OF APPROVED DESIGN MIXTURE FOR EACH TRUCK LOAD	
4.	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	[C]
5.	INSPECTION OF CONCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES PER ACI 318 26.5.2	[C]
6.	INSPECTION OF FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED	[P]

4 TESTING OF STRUCTURAL CONCRETE

A. SAMPLES FOR PREPARING STRENGTH TEST SPECIMENS OF EACH CONCRETE MIXTURE PLACED EACH DAY SHALL BE OBTAINED AT THE POINT OF PLACEMENT AND SHALL AT A MINIMUM BE TAKEN AS FOLLOWS: (A) AT LEAST ONCE A DAY (B) AT LEAST ONCE FOR EACH 100 CU. YD. OF CONCRETE (C) AT LEAST ONCE FOR EACH 5,000 S.F. OF SURFACE AREA FOR SLABS OR WALLS. SAMPLING OF CONCRETE SHALL BE IN ACCORDANCE WITH ASTM C172.
B. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIXTURE, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
C. SLUMP: ASTM C143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIXTURE. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
D AIR CONTENT: ASTM C231; TEST EACH COMPOSITE SAMPLE PER ASTM C231 (PRESSURE METHOD) FOR NORMAL-WEIGHT CONCRETE OR ASTM C173 (VOLUMETRIC METHOD) FOR LIGHT-WEIGHT CONCRETE.
E. TEMPERATURE: ASTM C1064; TEST EACH COMPOSITE SAMPLE AND AT 60-MINUTE INTERVALS. REQUIRED WHEN AIR TEMPERATURE IS 40°F AND BELOW OR 80°F AND ABOVE.
F. COLD WEATHER CURING: ASTM C1074; RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD WHEN A DAILY AVERAGE AIR TEMPERATURE OF 40°F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD.
G. COMPRESSION TEST SPECIMENS: ASTM C31;
CAST AND CURE FOUR (4) 6X12 CYLINDER SPECIMENS OR SIX (6) 4X8 CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE.
H. COMPRESSIVE STRENGTH TESTS: ASTM C39;
TEST ONE (1) 6X12 OR ONE (1) 4X8 SPECIMEN AT 7 DAYS
TEST TWO (2) 6X12 OR THREE (3) 4X8 SPECIMENS AT 28 DAYS
TEST ONE (1) 6X12 OR TWO (2) 4X8 SPECIMENS AT 56 DAYS IF 28-DAY TESTS DO NOT ACHIEVE THE SPECIFIED STRENGTH.
TEST SPECIMEN ACCEPTANCE CRITERIA TO BE IN ACCORDANCE WITH ACI 318

5 INSPECTION OF POST-INSTALLED ANCHORS [ANCHOR BOLTS, RODS & REINFORCING STEEL]

	ITEM	FREQUENCY
1.	MANUFACTURER, TYPE, DIAMETER, LENGTH AND FINISH OF ANCHOR	[C]
2.	MAXIMUM SPECIFIED IMPACT WRENCH TORQUE RATING MAINTAINED FOR SCREW ANCHORS	[C]
3.	ACCEPTABILITY OF BASE MATERIAL	[C]
4.	INSPECTION DURING ANCHOR INSTALLATION VERIFY DRILLING METHOD, HOLE DIMENSIONS, HOLE CLEANING, ANCHOR AND ADHESIVE PLACEMENT, ANCHOR EMBEDMENT, WRENCH TORQUE, EDGE DISTANCES AND SPACING.	[C]
5.	INSPECTION AFTER INSTALLATION OF ATTACHED ASSEMBLY VERIFY ANCHOR LOCATIONS, SPACING, EDGE DISTANCES, AND ANCHOR FLUSH WITH AND PERPENDICULAR TO THE RECEIVING SURFACE. VERIFY ANCHOR HEADS HAVE NOT BEEN CUT OFF AND THAT MANUFACTURER'S STAMP MARK IS READABLE AND HAS NOT BEEN DAMAGED OR OBSCURED.	[C]

NOTES:
A. MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) AND RELEVANT ICC-ES REPORTS SHALL BE USED ALONG WITH THE CONSTRUCTION DOCUMENTS TO DETERMINE COMPLIANCE.
B. CONTINUOUS INSPECTION OF ALL POST-INSTALLED ANCHORS SHALL BE REQUIRED, REGARDLESS OF WHETHER PERIODIC INSPECTION IS PERMITTED BY THE RELEVANT ICC-ES REPORTS.
C. PRIOR TO ANCHOR INSTALLATION, REVIEW AND VERIFY CONTRACTOR'S INSTALLATION PROCEDURE.
D. VERIFY THAT THE FULL CURE TIME AS OUTLINED IN THE GENERAL NOTES HAS ELAPSED PRIOR TO APPLICATION OF TORQUE OR LOAD TO ANCHOR.

6 INSPECTION OF MASONRY CONSTRUCTION

	ITEM	FREQUENCY
1.	VERIFY COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND APPROVED SUBMITTALS	[P]
2.	AT THE START OF MASONRY CONSTRUCTION, VERIFY:	
	A. PROPORTIONS OF SITE-PREPARED MORTAR	[P]
	B. CONSTRUCTION OF MORTAR JOINTS	[P]
	C. LOCATION OF REINFORCEMENT, CONNECTORS AND ANCHORAGES	[P]
3.	DURING MASONRY CONSTRUCTION, VERIFY:	
	A. SIZE AND LOCATION OF STRUCTURAL MEMBERS	[P]
	B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	[P]
	C. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT AND ANCHORAGES	[P]
	D. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)	[P]
4.	PRIOR TO GROUTING, VERIFY:	
	A. GROUT SPACE IS CLEAN	[P]
	B. PLACEMENT OF REINFORCING AND CONNECTORS AND ANCHORAGES	[P]
	C. PROPORTIONS OF SITE-PREPARED GROUT	[P]
	D. CONSTRUCTION OF MORTAR JOINTS	[P]
5.	VERIFY GROUT PLACEMENT	[C]

7 INSPECTION OF WOOD CONSTRUCTION

	ITEM	FREQUENCY
1.	ROOF DIAPHRAGMS:	
	A. SHEATHING THICKNESS AND GRADE	[P]
	B. NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES	[P]
	C. NAIL DIAMETER AND LENGTH	[P]
	D. NUMBER OF FASTENER LINES	[P]
	E. SPACING BETWEEN FASTENERS IN EACH LINE AND AT PANEL EDGES	[P]
2.	SHEARWALLS:	
	A. SHEATHING THICKNESS AND GRADE	[P]
	B. NOMINAL SIZE AND SPACING OF TYPICAL FRAMING MEMBERS AND AT ADJOINING PANEL EDGES	[P]
	C. NAIL DIAMETER AND LENGTH	[P]
	D. NUMBER OF FASTENER LINES	[P]
	E. SPACING BETWEEN FASTENERS IN EACH LINE AND AT PANEL EDGES	[P]
	F. LOCATION, SIZE AND TYPE OF HOLDOWNS	[P]
3.	NAILING, BOLTING, ANCHORING AND FASTENING OF:	
	A. DRAG STRUTS AND COLLECTORS	[P]
	B. HOLD-DOWNS	[P]

8 INSPECTION OF SOILS

	ITEM	FREQUENCY
1.	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE REQUIRED BEARING CAPACITY	[P]
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL	[P]
3.	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS	[P]
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF FILL	[P]
5.	PRIOR TO PLACEMENT OF CONTROLLED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY	[P]

NOTES:
A. THE APPROVED PROJECT GEOTECHNICAL REPORT SHALL BE USED ALONG WITH THE CONSTRUCTION DOCUMENTS TO DETERMINE COMPLIANCE.
B. DURING FILL PLACEMENT, THE SPECIAL INSPECTOR SHALL DETERMINE THAT PROPER MATERIALS AND PROCEDURES ARE USED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED PROJECT GEOTECHNICAL REPORT

REINFORCING BAR LAP SPLICE SCHEDULE

BAR SIZE	'TOP BAR' SPLICE LENGTH	'OTHER BAR' SPLICE LENGTH
#3	27"	21"
#4	35"	27"
#5	44"	34"
#6	52"	40"
#7	77"	59"
#8	86"	66"

NOTES:
1. 'TOP BARS' ARE HORIZONTAL BARS PLACED SUCH THAT 12" OF FRESH CONCRETE IS CAST BELOW THE BAR.
2. ALL BARS THAT ARE NOT 'TOP BARS' ARE 'OTHER BARS'.
3. REFER TO THE CONCRETE MATERIAL AND SPECIFICATION NOTES FOR ADDITIONAL INFORMATION AND REQUIREMENTS

STEEL BEAM SHEAR TAB CONNECTION SCHEDULE

BEAM SIZE	NUMBER OF ¾" Ø BOLTS	SHEAR TAB THK x LENGTH	FILLET WELD SIZE
W14	3	⅜"x9"	¼"
W16	4	⅜"x12"	¼"
W18	4	⅜"x12"	¼"

NOTES:
1. MINIMUM EDGE DISTANCE = 1½" FOR ALL CONNECTED PARTS.
2. BOLT SPACING = 3".
3. STANDARD HOLES TO BE PROVIDED IN SUPPORTED WEB.
4. SHORT-SLOTTED HOLES TO BE PROVIDED IN SHEAR TAB PLATES.
5. ALL BOLTS ARE TO BE A325.

HEADER SCHEDULE

MARK	HEADER	# OF TRIMMER STUDS	# OF KING STUDS
H1	(3) 2x10	1	2
H2	(3) 11⅞" LVL	2	3
H3	(3) 14" LVL	3	3

NOTES:
1. TYPICAL HEADERS, U.N.O.
2. RE: PLAN FOR MARK LOCATIONS.
3. GLUE AND NAIL PLY'S PER TYPICAL DETAIL THIS SHEET.
4. RE: 4/S1.2 FOR LATERAL STRAPS TO BE INSTALLED AT CORNERS OF OPENINGS LOCATED WITHIN LENGTH OF SHEARWALLS INDICATED ON PLAN.

LATERAL WOOD SHEARWALL SCHEDULE

MARK	APA RATED SHEATHING	SHEATHING NAILING	SILL PLATE ANCHORS	HOLDOWN	HOLDOWN ANCHOR	HOLDOWN END STUDS/POST
SW1	15/32" (EXTERIOR FACE)	8d @ 4" o.c. (PANEL EDGES) 8d @ 12" o.c. (FIELD)	⅝"ø x 1'-0" (HOOKED) @ 24" o.c. W/ BPS5/8-6 PLATE WASHERS (RE: NOTE 13 BELOW)	HDU5-SDS2.5 W/ (14) ¼" x 2½" SDS SCREWS INTO END STUDS	⅝"ø SSTB24	(2) 2x6
SW2	7/16"	8d @ 4" o.c. (PANEL EDGES) 8d @ 12" o.c. (FIELD)	⅝"ø x 1'-0" (HOOKED) @ 18" o.c. W/ BPS5/8-6 PLATE WASHERS (RE: NOTE 13 BELOW)	HDU8-SDS2.5 W/ (20) ¼" x 2½" SDS SCREWS INTO END POST	⅞"ø x 30" PAB7-30	6x4 POST

NOTES:
1. RE: PLAN FOR SHEARWALL MARK LOCATIONS AND EXTENTS.
2. RE: PLAN AND DETAILS FOR ADDITIONAL REQUIREMENTS AND DIMENSIONAL INFORMATION.
3. SHEARWALL STUDS SHALL BE 2x6 MINIMUM @ 16" MAX. o.c.
4. PANEL EDGES AT SHEARWALLS SHALL BE FULLY BLOCKED WITH 2x6 MATERIAL.
5. RE: 5/S1.2 FOR TYPICAL SHEARWALL HOLDOWN DETAIL.
6. RE: 6/S1.2 FOR TYPICAL SSTB HOLDOWN ANCHOR DETAIL.
7. RE: 7/S3.1 FOR PAB ANCHOR DETAIL.
8. SET HEIGHT OF SSTB ANCHORS USING EMBEDMENT MARK INDICATED TO ACCOMMODATE 2x SILL PLATE.
9. SECURE ANCHORS TO FORMWORK AND ADJACENT REINFORCING BARS TO PREVENT ANCHOR FROM BEING MOVED OR MISALIGNED DURING THE CONCRETE POUR.
10. INSTALL HOLDOWN HARDWARE AND ANCHORS IN CONFORMANCE WITH ALL SUPPLIER'S SPECIFICATIONS AND RECOMMENDATIONS, USING SUPPLIER PROVIDED FASTENERS AND ACCESSORIES.
11. HOLDOWN ANCHORS SHALL BE AS SPECIFIED AND SHALL BE CAST-IN-PLACE. POST-INSTALLED WEDGE OR EPOXIED ANCHOR ALTERNATES ARE NOT ACCEPTABLE.
12. RE: 4/S1.2 FOR ADDITIONAL LATERAL STRAPS TO BE INSTALLED AT CORNERS OF OPENINGS LOCATED WITHIN LENGTH OF SHEARWALLS.
13. RE: 7/S1.2 FOR SILL ANCHOR PLATE WASHER INSTALLATION AT LATERAL SHEARWALLS.
14. SHEATHING SHALL BE PROVIDED IN 4x8 PANELS AND INSTALLED WITH LONG DIMENSION ORIENTED HORIZONTALLY.

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Project Number 25-001

02.11.2025

Rev	Description	Date

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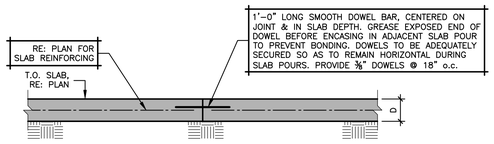
STRUCTURAL SPECIAL INSPECTIONS & SCHEDULES

S1.1

BOXED NOTES INDICATE TYPICAL NOTES, U.N.O.

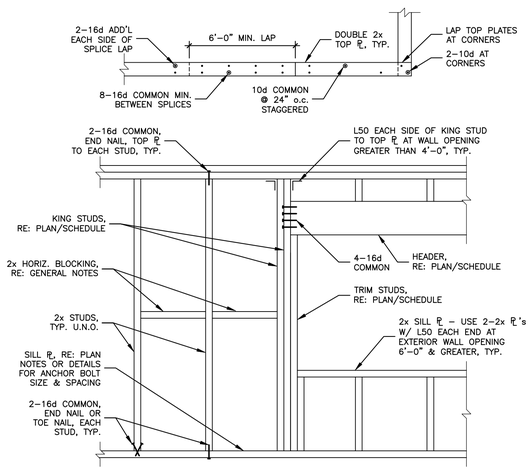


TYPICAL CONSTRUCTION JOINT

NOTES:
1. JOINTS MUST BE TOOLED OR SAW CUT AS SOON AS THE SLAB CAN SUPPORT EQUIPMENT.
2. LOCATE AT MAXIMUM SPACING OF APPROXIMATELY 12'-0" EACH DIRECTION AND AT COLUMN CENTERLINES AND RE-ENTRANT CORNERS.

TYPICAL CONTROL JOINT

1 SLAB-ON-GRADE CONSTRUCTION & CONTROL JOINT 3/4"=1'-0"



2 TYPICAL WOOD STUD WALL FRAMING DETAIL N.T.S.

NOTES: 1. HEADERS SHALL BE 2-2x8 MINIMUM, TYP. U.N.O.
2. 2x SOLID BLOCKING REQUIRED AT CEILING LINES

SAWN LUMBER (2" NOM. THICKNESS) CONNECTION SCHEDULE

PLY TO PLY CONNECTION	2 PLY	3 PLY
2-ROWS 10d NAILS @ 12" o.c.	ONE FACE	EACH FACE

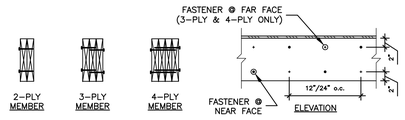
FOR COLUMNS OVER 3-PLY, ATTACH EACH ADDITIONAL PLY W/ 2 ROWS OF 10d NAILS @ 12" o.c.

LVL 1-3/4" THICK, MEMBER DEPTH </= 14" CONNECTION SCHEDULE

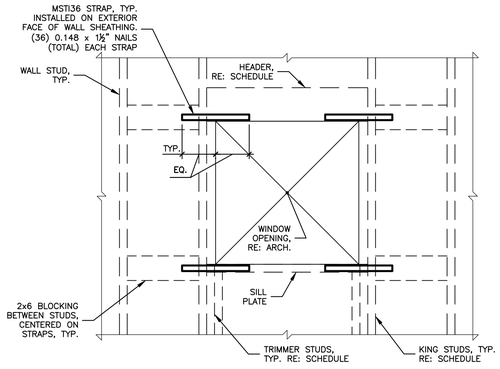
PLY TO PLY CONNECTION	2 PLY	3 PLY	4 PLY
2-ROWS 12d NAILS @ 12" o.c.	ONE FACE	EACH FACE	——
2-ROWS ¼"x6" SDS SCREWS @ 24" o.c.	——	——	EACH FACE

LVL 1-3/4" THICK, MEMBER DEPTH > 14" CONNECTION SCHEDULE

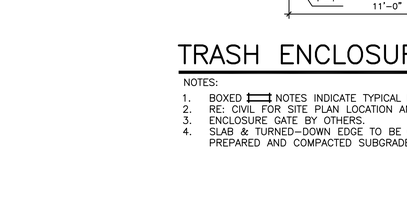
PLY TO PLY CONNECTION	2 PLY	3 PLY	4 PLY
3-ROWS 16d NAILS @ 12" o.c.	ONE FACE	EACH FACE	——
3-ROWS ¼"x6" SDS SCREWS @ 24" o.c.	——	——	EACH FACE



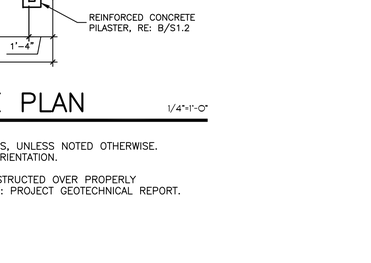
3 TYPICAL MULTIPLE WOOD PLY CONNECTIONS N.T.S.



4 STRAPS AT WINDOW OPENING WITHIN LENGTH OF SHEARWALL N.T.S.

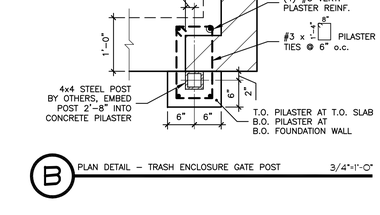


5 TYPICAL SHEARWALL HOLDOWN DETAIL 3/4"=1'-0"

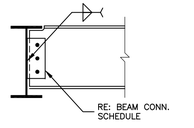


6 TYPICAL SSTB HOLDOWN ANCHOR 1-1/2"=1'-0"

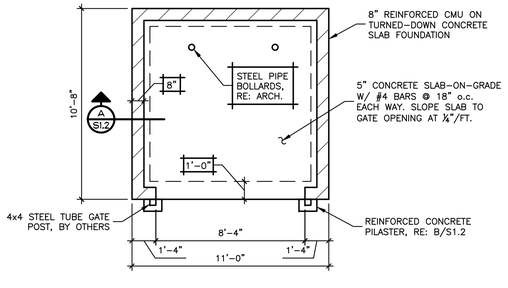
7 SILL PLATE ANCHOR ROD PLATE WASHER AT SHEARWALLS 1-1/2"=1'-0"



8 TYPICAL STEEL BEAM TO HSS COLUMN CONNECTION 3/4"=1'-0"

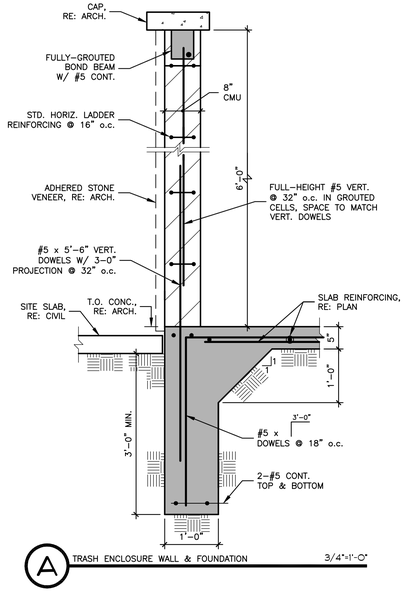


9 TYPICAL STEEL BEAM TO BEAM CONNECTION 3/4"=1'-0"



TRASH ENCLOSURE PLAN 1/4"=1'-0"

NOTES:
1. BOXED ⟺ NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. RE: CIVIL FOR SITE PLAN LOCATION AND ORIENTATION.
3. ENCLOSURE GATE BY OTHERS.
4. SLAB & TURNED-DOWN EDGE TO BE CONSTRUCTED OVER PROPERLY PREPARED AND COMPACTED SUBGRADE, RE: PROJECT GEOTECHNICAL REPORT.



A TRASH ENCLOSURE WALL & FOUNDATION 3/4"=1'-0"

B PLAN DETAIL - TRASH ENCLOSURE GATE POST 3/4"=1'-0"

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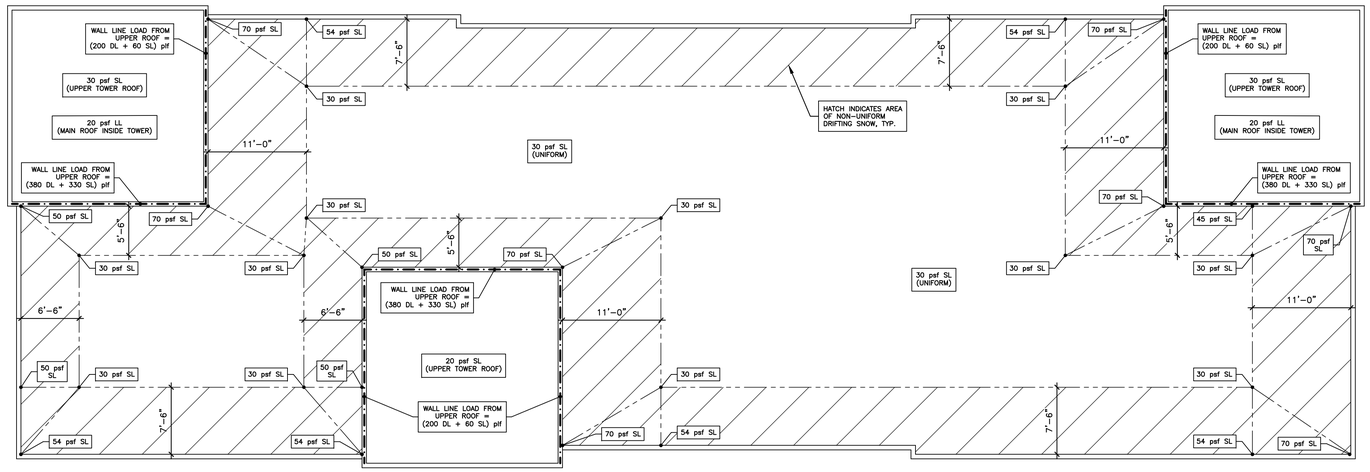
TYPICAL DETAILS & TRASH ENCLOSURE

S1.2

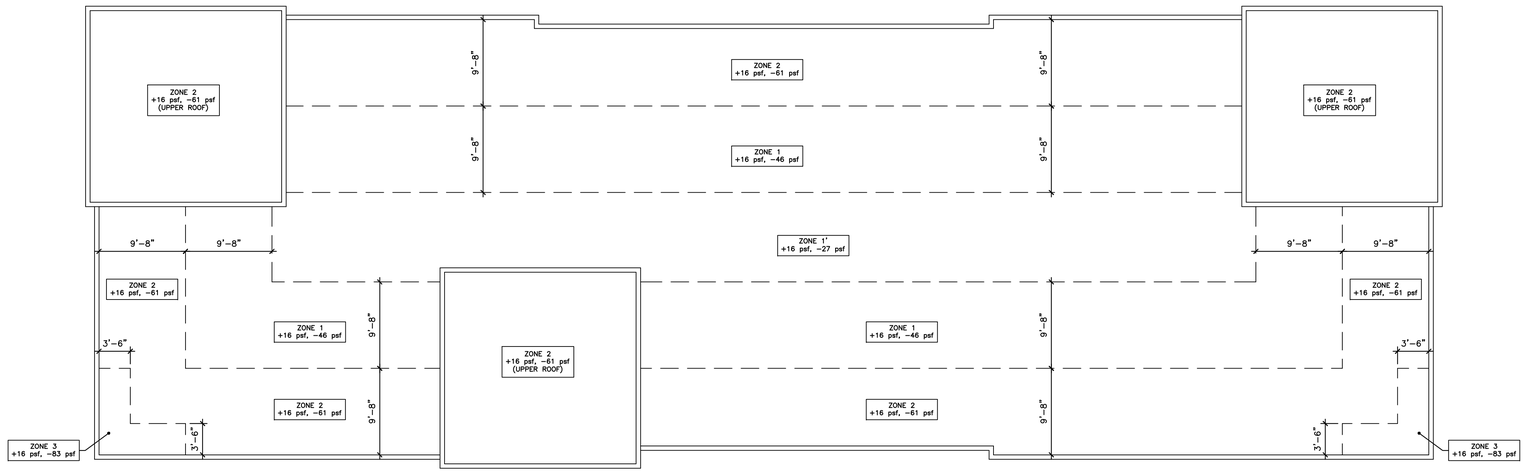


ROOF GRAVITY DESIGN LOAD DIAGRAM

N.T.S.

NOTES:

1. 'DL' INDICATES DEAD LOAD. 'LL' INDICATES LIVE LOAD. 'SL' INDICATES SNOW LOAD.
2. UNIFORM SUPERIMPOSED ROOF DL = 15 psf TO BE INCLUDED.
3. DRIFTING SNOW LOADS IN HATCHED AREAS VARY UNIFORMLY BETWEEN VALUES INDICATED.
4. RE: GENERAL NOTES ON SHEET S1.0 FOR ADDITIONAL DESIGN INFORMATION.
5. ALL DESIGN LOADS INDICATED IN THE DRAWINGS SHALL BE USED, CONSIDERING ALL APPLICABLE LOAD COMBINATIONS FROM IBC SECTION 1605.



ROOF WIND DESIGN LOAD DIAGRAM

N.T.S.

NOTES:

1. VALUES REPRESENT TOTAL (NOT NET) WIND LOADS.
2. VALUES SHOWN ARE ULTIMATE (NOT ASD) LOADS, COMPUTED PER ASCE 7-16.
3. (+) VALUES INDICATE DOWNWARD WIND LOADS. (-) VALUES INDICATED UPWARD WIND LOADS.
4. RE: GENERAL NOTES ON SHEET S1.0 FOR ADDITIONAL DESIGN INFORMATION.
5. RE: DETAIL 1/S4.1 FOR ADDITIONAL VERTICAL PARAPET KICKER WIND LOAD REACTIONS.
6. WIND LOADS SHALL BE COMBINED WITH ALL OTHER DESIGN LOADS INDICATED IN THE DRAWINGS, CONSIDERING ALL APPLICABLE LOAD COMBINATIONS FROM IBC SECTION 1605.

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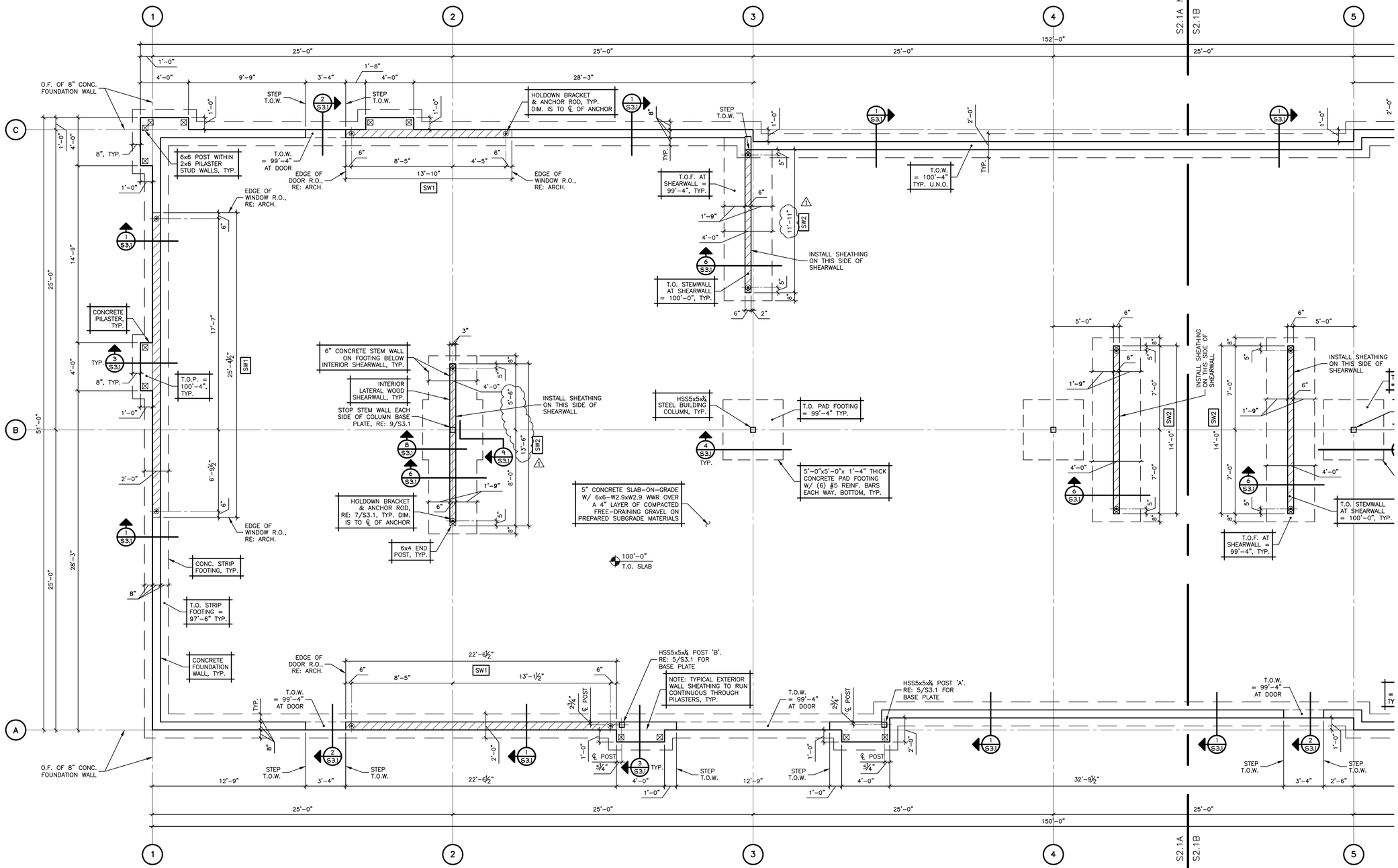
ROOF DESIGN LOAD DIAGRAMS

S1.3



FOUNDATION/FLOOR PLAN – NORTH

1/4"=1'-0"

NOTES:

1. BOXED NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. TOP OF SLAB ELEVATION = 100'-0". RE: CIVIL PLANS FOR U.S.G.S. ELEVATION.
3. RE: GENERAL NOTES AND DETAIL FOR TYPICAL SLAB-ON-GRADE CONSTRUCTION AND CONTROL JOINT REQUIREMENTS.
4. TOP OF CONCRETE WALL WALL (T.O.W.) = 100'-4", UNLESS NOTED OTHERWISE.
5. TOP OF PILASTER ELEVATION (T.O.P.) = 100'-4", UNLESS NOTED OTHERWISE.
6. ALL FOOTINGS AND FLOOR SLABS SHALL BE CONSTRUCTED OVER PROPERLY CONDITIONED AND COMPACTED SUBGRADE MATERIALS. THE EXISTING FILL MATERIAL WITHIN THE BUILDING FOOTPRINT SHOULD BE REMOVED TO FIRM EOLIAN SOILS, CONDITIONED AND COMPACTED AS ENGINEERED FILL AS SPECIFIED IN THE PROJECT GEOTECHNICAL REPORT. ENGINEERED FILL SHOULD EXTEND LATERALLY 5 FEET OR MORE BEYOND THE FOUNDATION LIMITS AND COVER THE LIMITS OF THE APPURTENANCES THAT ARE ADJACENT TO THE BUILDING. REFER TO THE PROJECT GEOTECHNICAL REPORT FOR ADDITIONAL SUBGRADE PREPARATION REQUIREMENTS AND RECOMMENDATIONS.
7. COMPLETED FOUNDATION EXCAVATION SHALL BE OBSERVED BY A QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION CONSTRUCTION TO VERIFY SUBGRADE CONDITIONS SPECIFIED IN THE PROJECT GEOTECHNICAL REPORT.
8. CENTER ALL PAD FOOTINGS UNDER COLUMNS, UNLESS NOTED OTHERWISE.
9. CENTER ALL STRIP FOOTINGS UNDER FOUNDATION WALLS, UNLESS NOTED OTHERWISE.
10. RE: ARCHITECTURAL AND M/E/P DRAWINGS FOR REQUIRED SLEEVES OR BLOCKOUTS IN FOUNDATION WALLS AND FLOOR SLABS.
11. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

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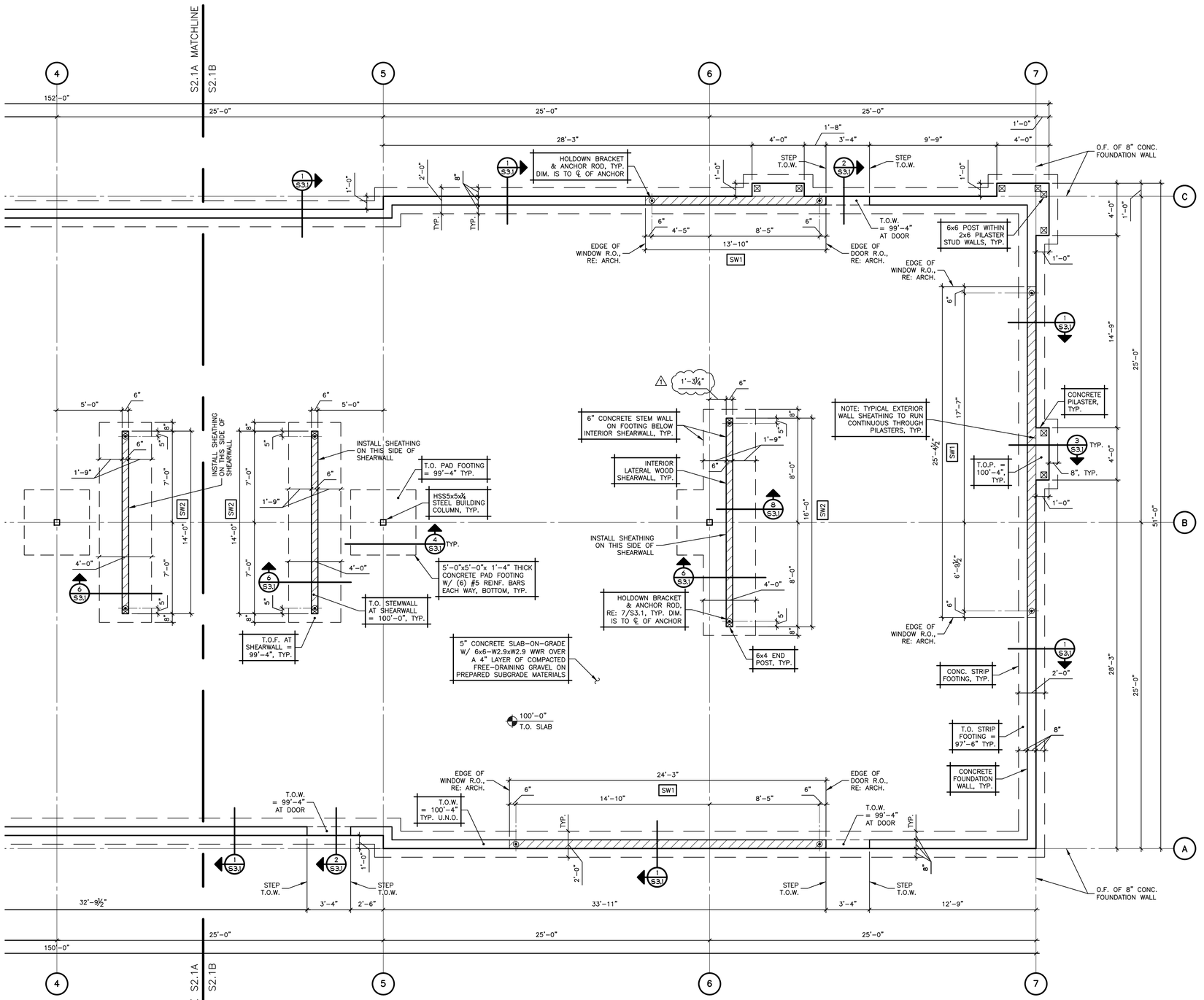
Rev	Description	Date
1	SHEARWALL REVISIONS	03.12.25

FOUNDATION / FLOOR PLAN - NORTH

S2.1A



FOUNDATION/FLOOR PLAN – SOUTH

1/4"=1'-0"

NOTES:

1. BOXED ⟷ NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. TOP OF SLAB ELEVATION = 100'-0". RE: CIVIL PLANS FOR U.S.G.S. ELEVATION.
3. RE: GENERAL NOTES AND DETAIL FOR TYPICAL SLAB-ON-GRADE CONSTRUCTION AND CONTROL JOINT REQUIREMENTS.
4. TOP OF CONCRETE WALL WALL (T.O.W.) = 100'-4", UNLESS NOTED OTHERWISE.
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7. COMPLETED FOUNDATION EXCAVATION SHALL BE OBSERVED BY A QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION CONSTRUCTION TO VERIFY SUBGRADE CONDITIONS SPECIFIED IN THE PROJECT GEOTECHNICAL REPORT.
8. CENTER ALL PAD FOOTINGS UNDER COLUMNS, UNLESS NOTED OTHERWISE.
9. CENTER ALL STRIP FOOTINGS UNDER FOUNDATION WALLS, UNLESS NOTED OTHERWISE.
10. RE: ARCHITECTURAL AND M/E/P DRAWINGS FOR REQUIRED SLEEVES OR BLOCKOUTS IN FOUNDATION WALLS AND FLOOR SLABS.
11. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

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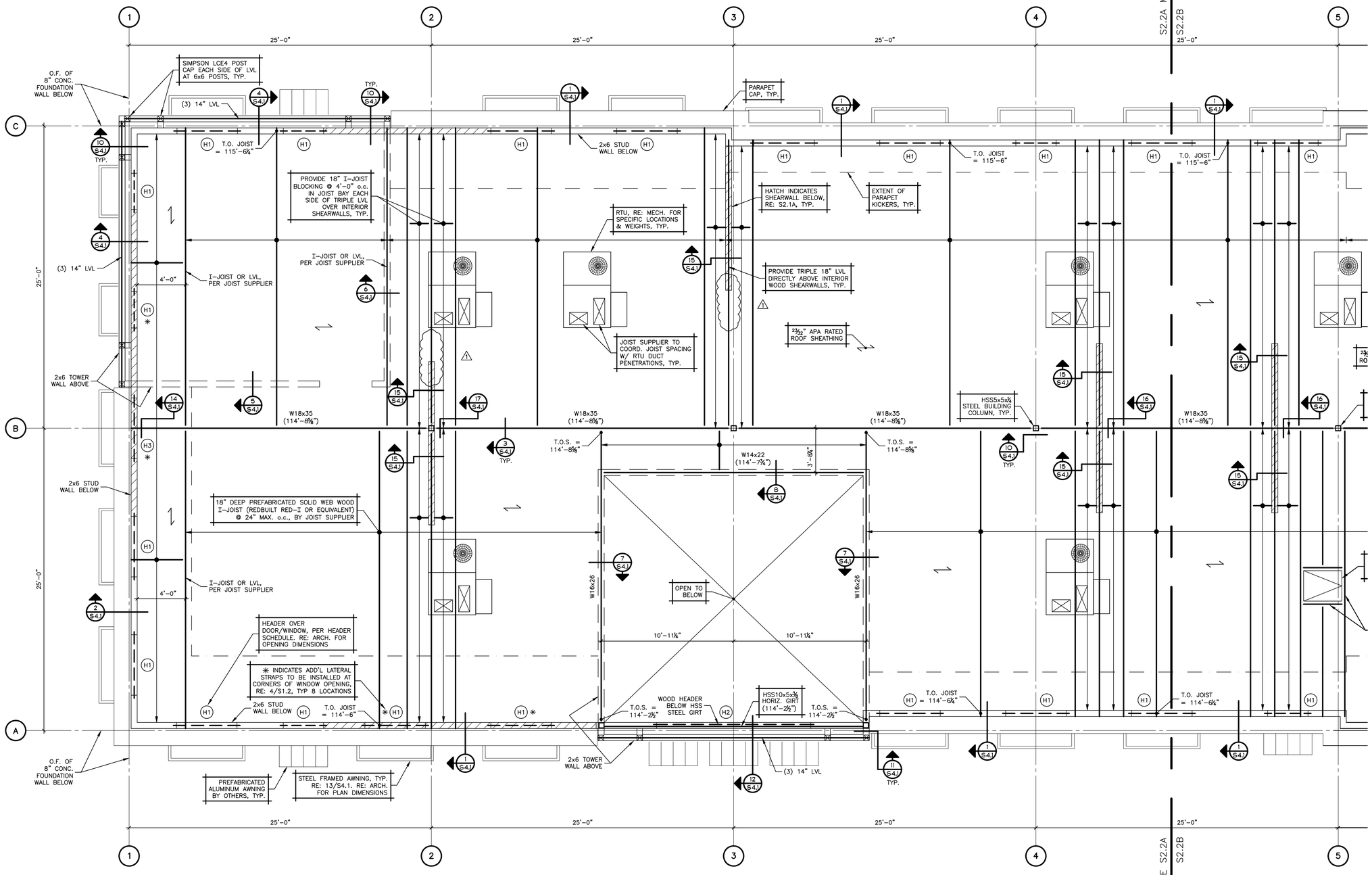
1	SHEARWALL REVISIONS	03.12.25

FOUNDATION / FLOOR PLAN - SOUTH

S2.1B



MAIN ROOF FRAMING PLAN – NORTH

1/4"=1'-0"

NOTES:

1. BOXED NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. INDICATES SPAN DIRECTION OF ROOF SHEATHING. RE: GENERAL NOTES FOR FASTENING REQUIREMENTS.
3. TOP OF LEVEL STEEL BEAM ELEVATION NOTED THUS: (XXX'-XX").
4. T.O.S. INDICATES TOP OF STEEL ELEVATION OF SLOPING STEEL BEAM. SLOPE TOP OF BEAMS BETWEEN ELEVATIONS NOTED.
5. COORDINATE SIZE AND LOCATION OF MECHANICAL UNITS AND ROOF PENETRATIONS FOR MECHANICAL DUCTWORK WITH ARCHITECT, MECHANICAL ENGINEER AND SUPPLIER.
6. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

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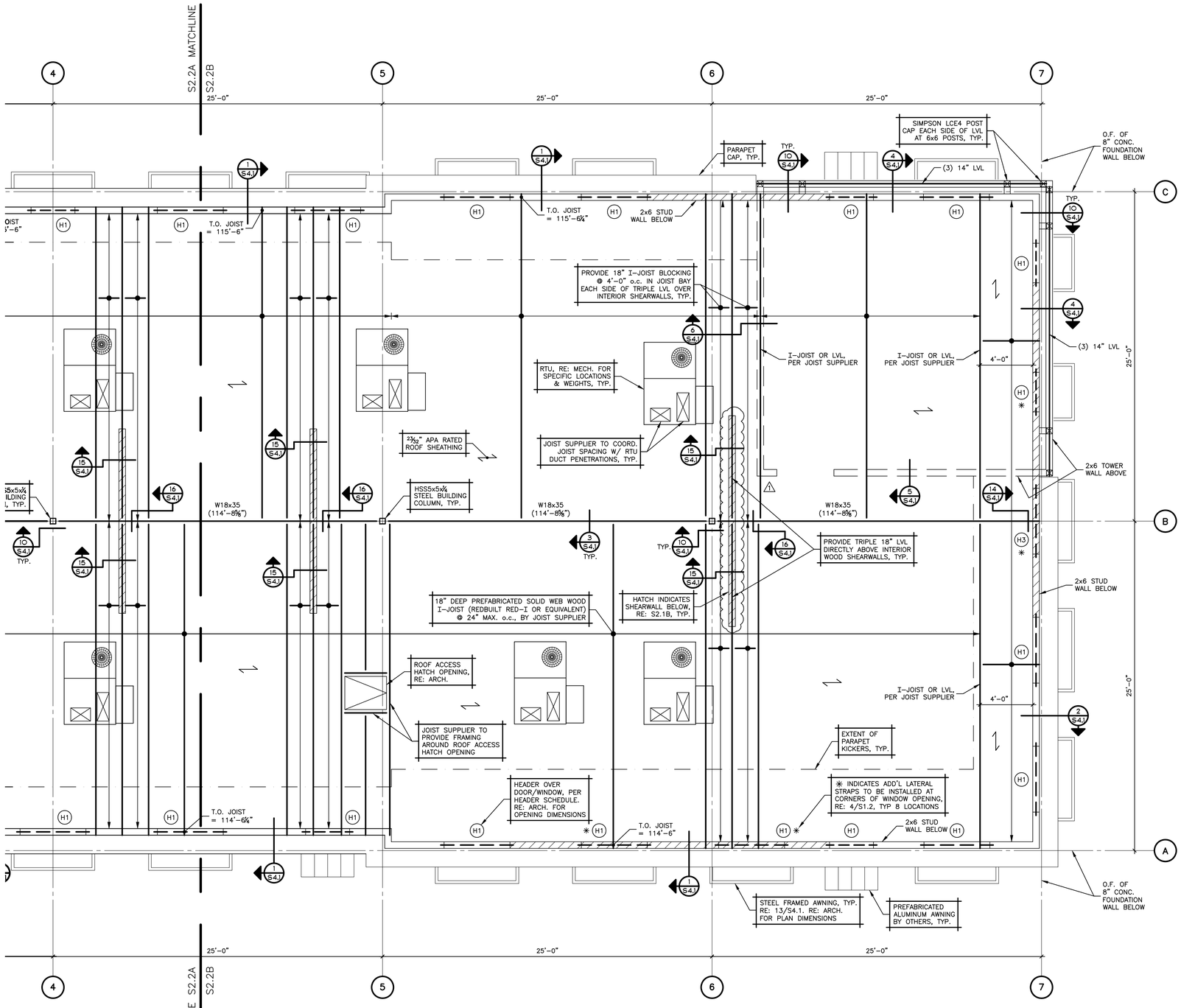
MAIN ROOF FRAMING PLAN - NORTH

S2.2A



MAIN ROOF FRAMING PLAN – SOUTH

1/4"=1'-0"

NOTES:

1. BOXED NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. INDICATES SPAN DIRECTION OF ROOF SHEATHING. RE: GENERAL NOTES FOR FASTENING REQUIREMENTS.
3. TOP OF LEVEL STEEL BEAM ELEVATION NOTED THUS: (XXX'-XX").
4. T.O.S. INDICATES TOP OF STEEL ELEVATION OF SLOPING STEEL BEAM. SLOPE TOP OF BEAMS BETWEEN ELEVATIONS NOTED.
5. COORDINATE SIZE AND LOCATION OF MECHANICAL UNITS AND ROOF PENETRATIONS FOR MECHANICAL DUCTWORK WITH ARCHITECT, MECHANICAL ENGINEER AND SUPPLIER.
6. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

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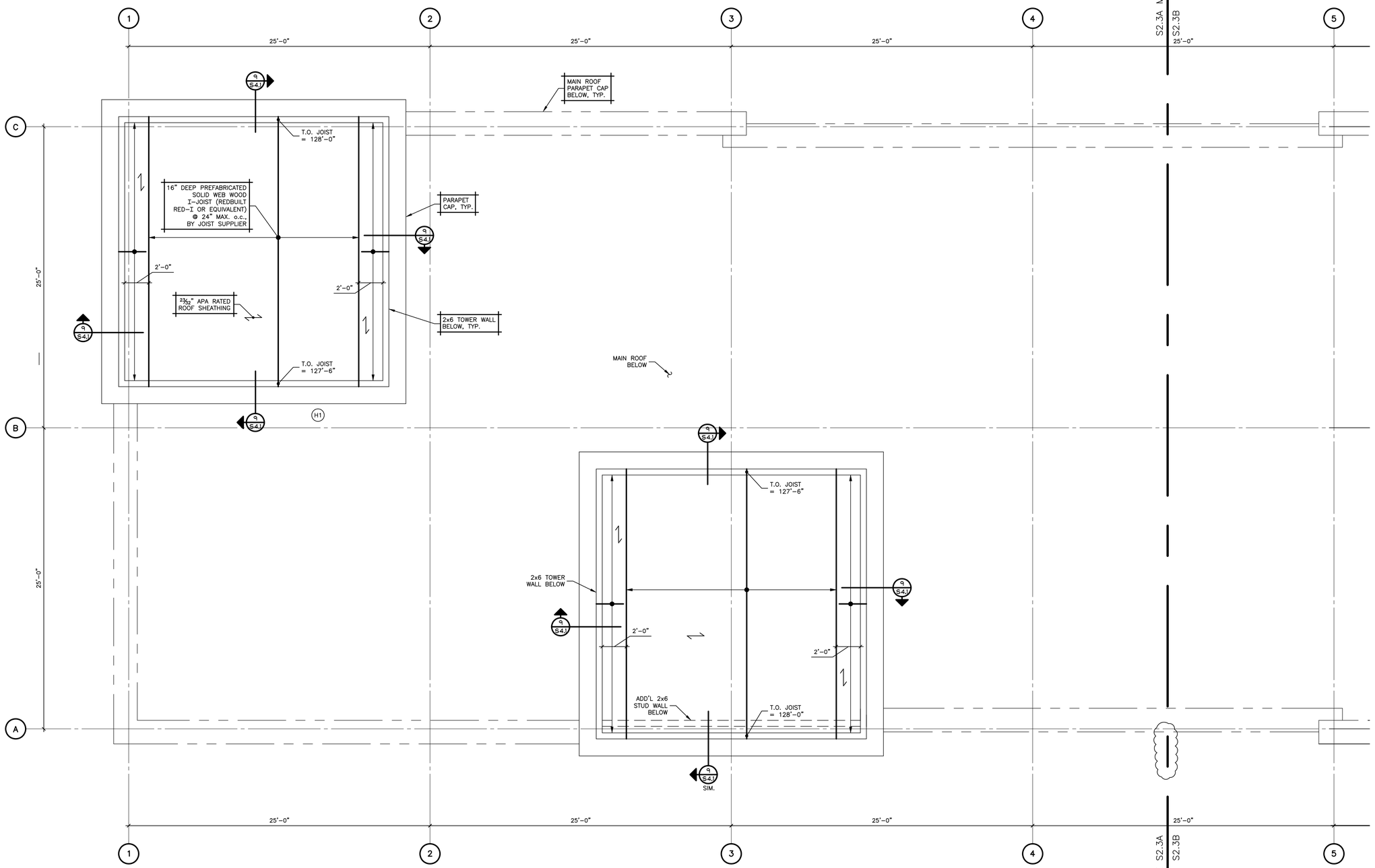
1	SHEARWALL REVISIONS	03.12.25

MAIN ROOF FRAMING PLAN - SOUTH

S2.2B



UPPER ROOF FRAMING PLAN – NORTH

1/4"=1'-0"

NOTES:

1. BOXED ⟷ NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. ⟋ INDICATES SPAN DIRECTION OF ROOF SHEATHING. RE: GENERAL NOTES FOR FASTENING REQUIREMENTS.
3. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

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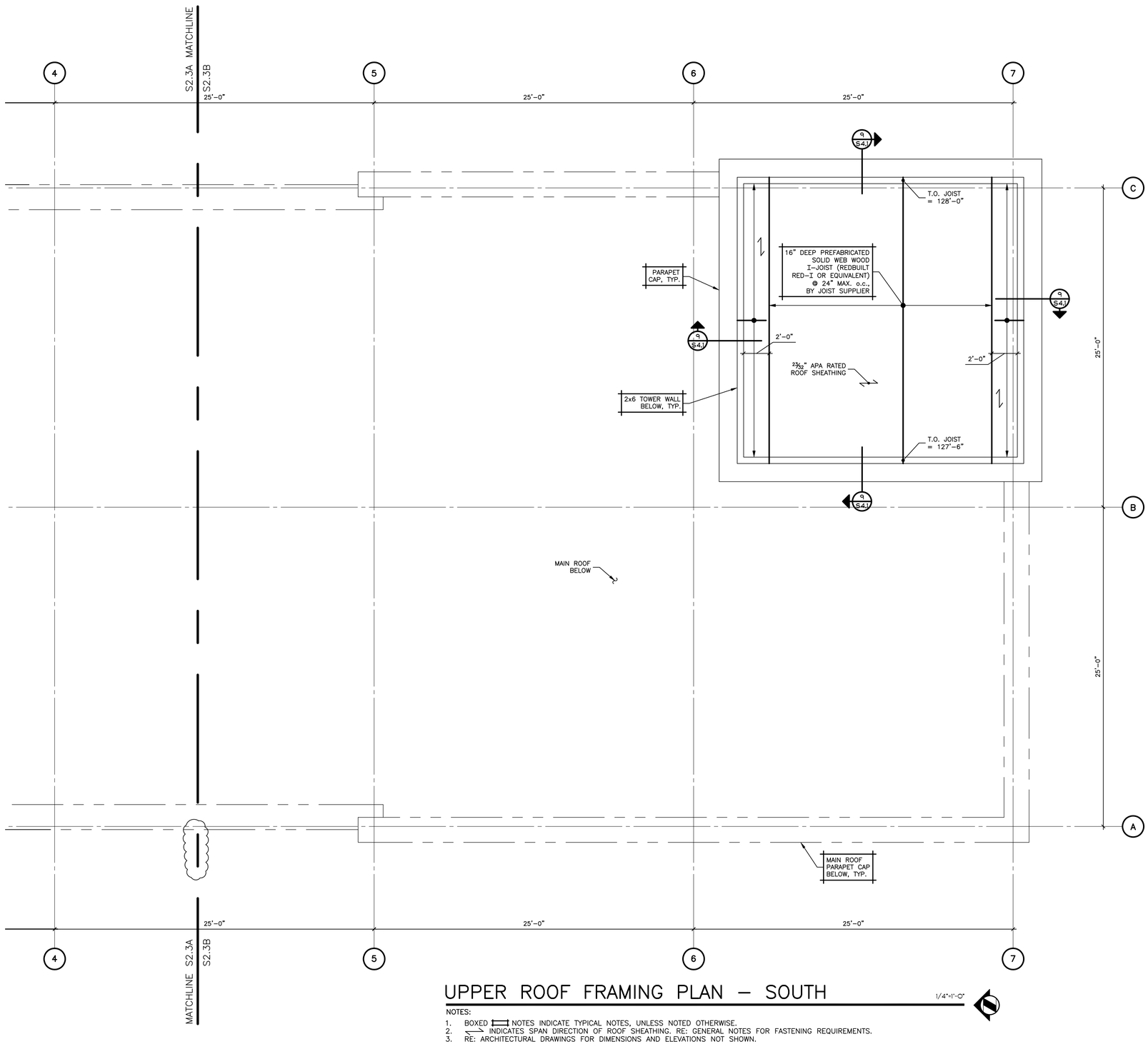
UPPER ROOF FRAMING PLAN - NORTH

S2.3A



UPPER ROOF FRAMING PLAN – SOUTH

1/4"=1'-0"

NOTES:

1. BOXED ⟦ NOTES INDICATE TYPICAL NOTES, UNLESS NOTED OTHERWISE.
2. ⟋ INDICATES SPAN DIRECTION OF ROOF SHEATHING. RE: GENERAL NOTES FOR FASTENING REQUIREMENTS.
3. RE: ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS NOT SHOWN.

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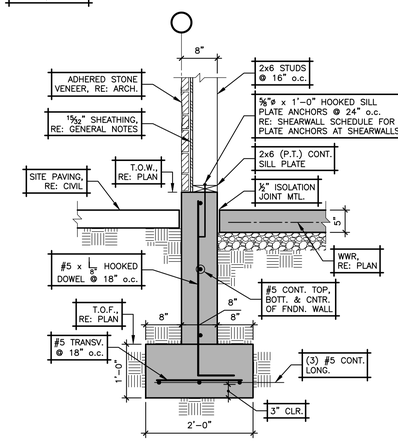
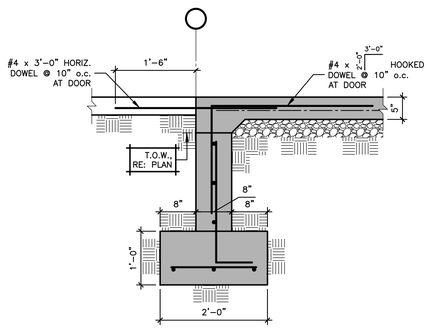
UPPER ROOF FRAMING PLAN - SOUTH

S2.3B

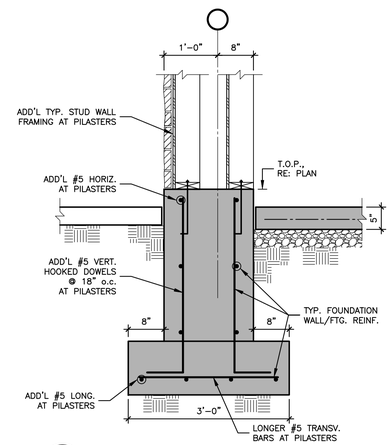
BOXED NOTES INDICATE TYPICAL NOTES, U.N.O.



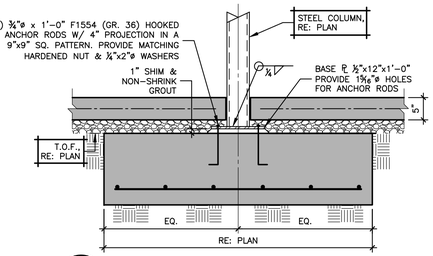
1 3/4"=1'-0"



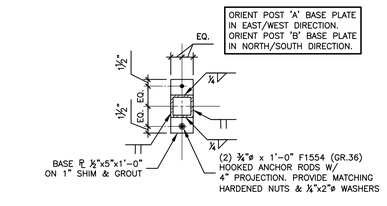
2 3/4"=1'-0"



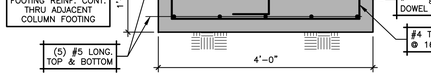
3 3/4"=1'-0"



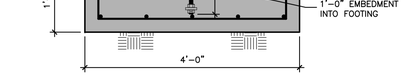
4 3/4"=1'-0"



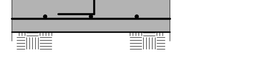
5 BASE PL DETAIL – HSS POST 'A' & 'B' 3/4"=1'-0"



6 3/4"=1'-0"



7 3/4"=1'-0"



8 3/4"=1'-0"

9 3/4"=1'-0"

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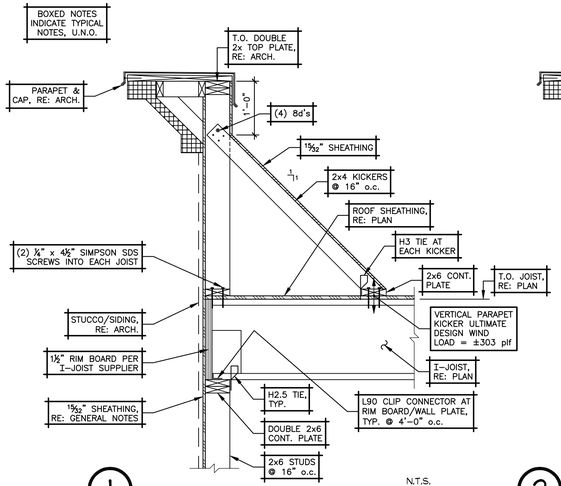
FOUNDATION/
FLOOR DETAILS

S3.1



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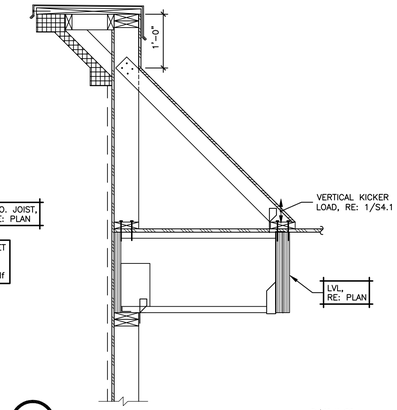
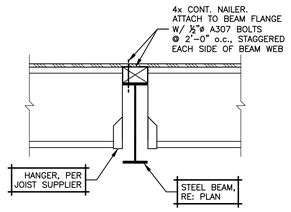
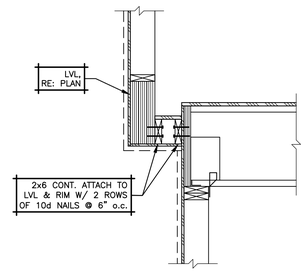


2 3/4"=1'-0"



3 3/4"=1'-0"



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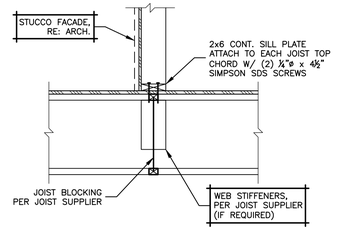

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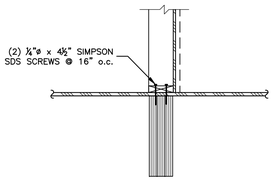

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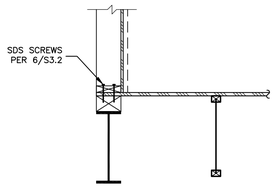

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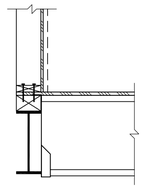

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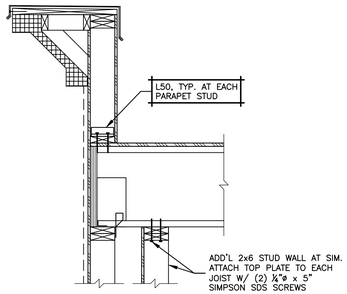

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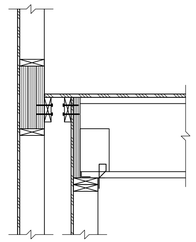

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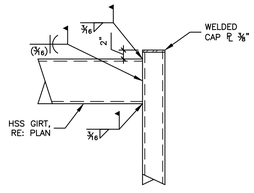

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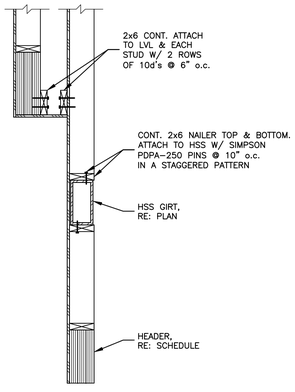

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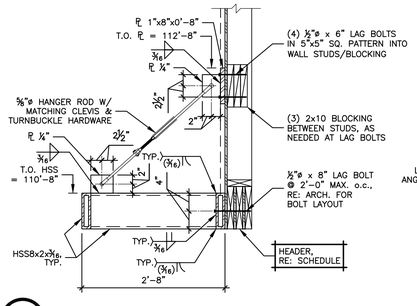

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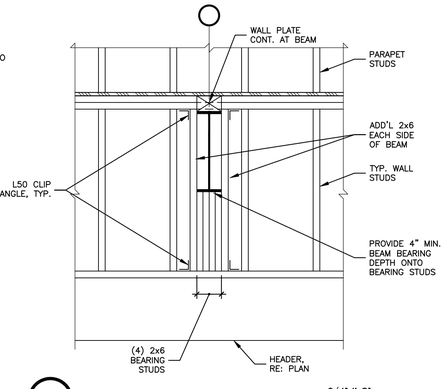

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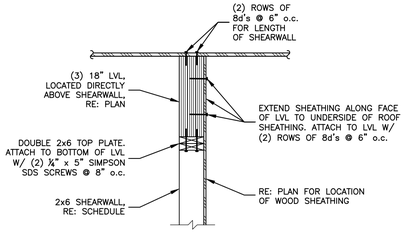

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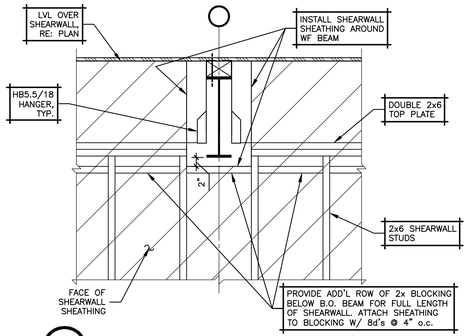

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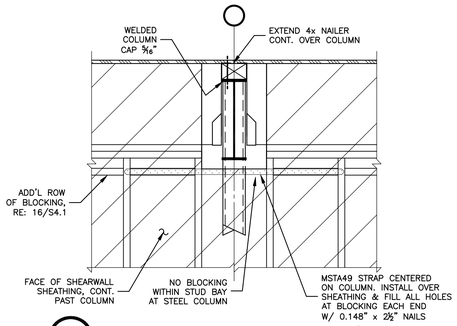

17 3/4"=1'-0"

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Project Number	25-001
	02.11.2025

Rev	Description	Date

FRAMING DETAILS

S4.1